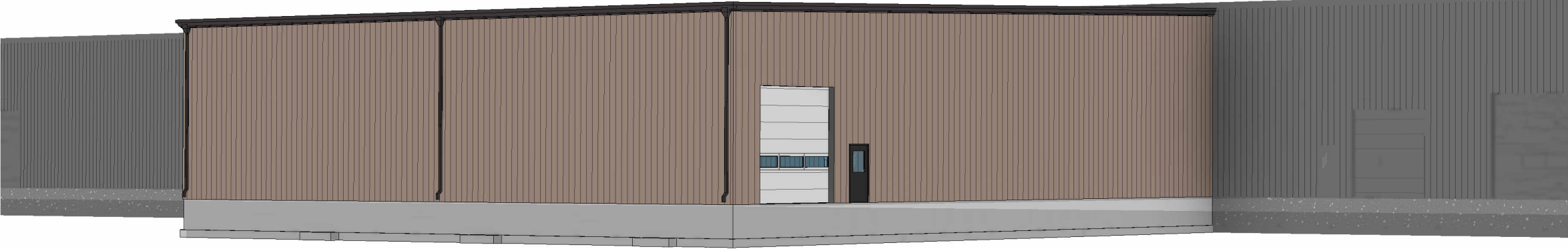


2025 BUILDING ADDITION
UPI MANUFACTURING

587W35670 GODFREY LN
EAGLE, WI 53119
NOVEMBER 25, 2025



PROPOSED COMPLETED ELEVATION

| OWNER |
|-----------------------------------------------------------------------------------------------------------------|
| DRETZKA INVESTMENTS LLC C/O JEFF DRETZKA W33454375 CONNEMARA DR DOUGMAN, WI 53118 P. (608) 846-4711 |

| INDEX OF DRAWINGS | |
|-------------------|-------------------------|
| SHEET NO. | DESCRIPTION |
| GENERAL - | |
| G1.0 | TITLE SHEET |
| CIVIL - | |
| C1.0 | EXISTING PLAN |
| C2.0 | PROPOSED SITE PLAN |
| C3.0 | EROSION CONTROL PLAN |
| C4.0 | EROSION CONTROL DETAILS |

| INDEX OF DRAWINGS | |
|-------------------|--------------------------------|
| SHEET NO. | DESCRIPTION |
| ARCHITECTURAL - | |
| A-001 | FIRST FLOOR ARCHITECTURAL CODE |
| A-100 | FIRST FLOOR PLAN |
| A-200 | REFLECTED CEILING PLAN |
| A-300 | ROOF PLAN |
| A-400 | EXTERIOR ELEVATIONS |
| A-500 | BUILDING SECTIONS |
| A-600 | WALL SECTIONS |
| A-610 | DOOR & WINDOW SCHEDULES |
| STRUCTURAL - | |
| S001 | STRUCTURAL NOTES |
| S100 | FOUNDATION PLAN |
| S110 | BASE PLATE DETAILS |
| S111 | FOUNDATION DETAILS |
| S200 | ROOF FRAMING PLAN |
| MECHANICAL - | DESIGN BUILT |
| PLUMBING - | DESIGN BUILT |
| ELECTRICAL - | DESIGN BUILT |

BUILDING DESIGN CRITERIA

- **CODE COMPLIANCE PER 2025 WISCONSIN**
COMMERCIAL BUILDING CODE (WCB) (2021 IBC - CODES)
ENERGY COMPLIANCE PER IECC 2021
- **BUILDING ENVELOPE REQUIREMENTS:**
CODE COMPLIANCE PER 2025 WISCONSIN
COMMERCIAL BUILDING CODE (WCB) (2021 IBC - CODES)
CONDITIONED SPACE PER ASHRAE 90.1 (2019).
MTL BLDG ROOF (U=0.035) 5PS 363.0402
MTL BLDG WALL (U=0.050) 5PS 363.0402
DOORS, SWINGING (U=0.37) 5PS 363.0403
DOORS, NON SWINGING (U=0.31)
- **PROVIDED THERMAL ENVELOPE:**
MTL BUILDING ROOF R25+RII LS, S5R (U=0.031)
MTL BUILDING WALL R30 (0.052) W/ THERMAL SPACER
BLOCKS
- **OCCUPANCY TYPE**
F2 - FACTORY (METAL PARTS ASSEMBLY) = 23,000 SF
- **CONSTRUCTION CLASSIFICATION - IIB**
METAL FRAME BUILDING
- **BUILDING ALLOWABLE AREA**
 - IBC 506.2 = 23,000 SF NON SPRINKLED
 - FRONTAGE INCREASE = 75% * 23,000 SF = 17,250 SF
- **OCCUPANT LOAD**
FACTORY F-2 WITH 18 OCCUPANTS

ABBREVIATIONS

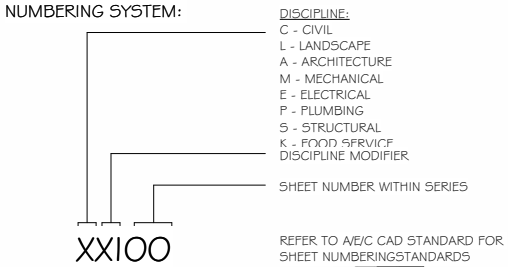
- EOP = EDGE OF PAVEMENT
BOC = BACK OF CURB
EOSW = EDGE OF SIDEWALK
TOF = TOP OF FOOTING
FFE = FIRST FLOOR ELEVATION
TOW = TOP OF WALL
SFE = SECOND FLOOR ELEVATION
ELEV. = ELEVATION
RO = ROUGH OPENING
BM = BENCHMARK
SQ. FT. = SQUARE FEET
DIA. = DIAMETER
TYP. = TYPICAL
HM = HOLLOW METAL
SS = STAINLESS STEEL
ALUM. = ALUMINUM
IBC = INTERNATIONAL BUILDING CODE
WD = WOOD
MTL = METAL
HSS = HOLLOW STEEL STRUCTURE
STL = STEEL
OFOI - OWNER FURNISHED OWNER INSTALLED
- MFG. = MANUFACTURER
DIM. = DIMENSION
SPF = SPRUCE PINE FUR
DF = DOUGLAS FIR
SP = SOUTHERN PINE
CONC. = CONCRETE
WWF = WIRE WELDED FABRIC
OC = ON CENTER
EW = EACH WAY
EF = EACH FACE
WH = WATER HEATER
DW = DISHWASHER
REF. = REFRIGERATOR
FRZ. = FREEZER
FTG. = FOOTING
PC = PRECAST
OHD = OVERHEAD DOOR
TJO = TOP OF
GALV. = GALVANIZED
BRG. = BEARING
OH = OVERHANG

SEAL

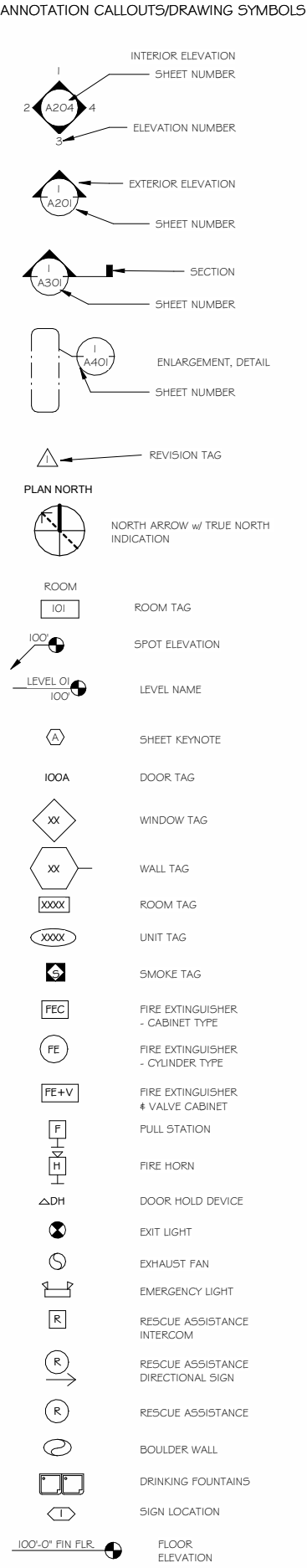


I HEREBY CERTIFY THAT THIS PLAN SPECIFICATION OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM DULY REGISTERED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF WISCONSIN.

INDEX OF DRAWINGS

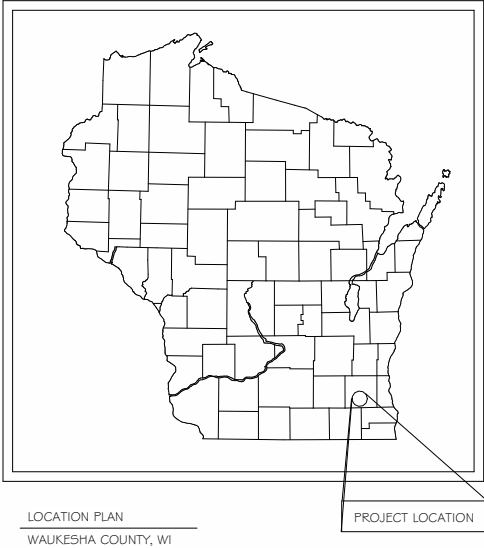


ARCHITECTURAL LEGEND



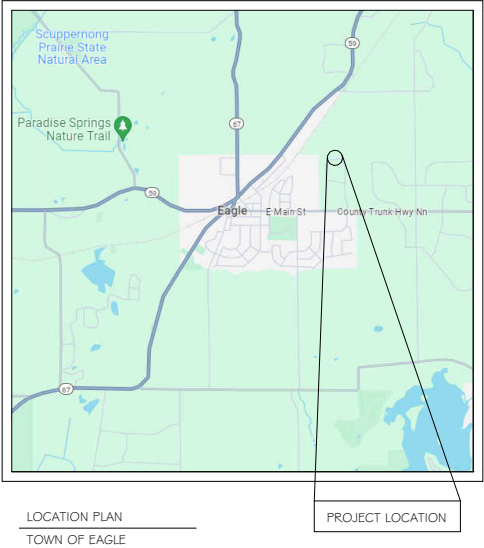
| | |
|--------------------------|---------------------------|
| SUBMITTAL TYPE | ADDITION |
| PRIMARY OCCUPANCY TYPE | LOW HAZZARD FACTORY (F-2) |
| SECONDARY OCCUPANCY TYPE | N/A |
| OCCUPANCY SEPARATIONS | N/A |

| | |
|----------------------|---------------|
| PROJECT AREA | 9,000 SQ. FT. |
| FLOOR LEVELS | 1 |
| CONSTRUCTION CLASS | IIB |
| SPRINKLER PROTECTION | NONE |



LOCATION PLAN
WAUKESHA COUNTY, WI

PROJECT LOCATION



LOCATION PLAN
TOWN OF EAGLE

PROJECT LOCATION

SHEET NAME:

TITLE SHEET

ADDRESS:

587W35670
GODFREY LN
EAGLE, WI 53119

PROJECT:

2025 BUILDING ADDITION
UPI MANUFACTURING

ROUSSEV
ENGINEERING
SOLUTIONS, LLC

7563 Driftless Rdg Wy
Verona, WI 53593
608-620-3036 (Office) • svet@rousseveengineering.com

ADVANCED
BUILDING
CORPORATION

3802 KIPP ST.
MADISON, WI 53718

PROJECT NO: 0924-25
PLOT DATE: 11/1/25
PLOT BY: SSR
PLOT SCALE: As indicated

SHEET # G1.0

BID DOCUMENTS

SITE DEVELOPMENT DATA:

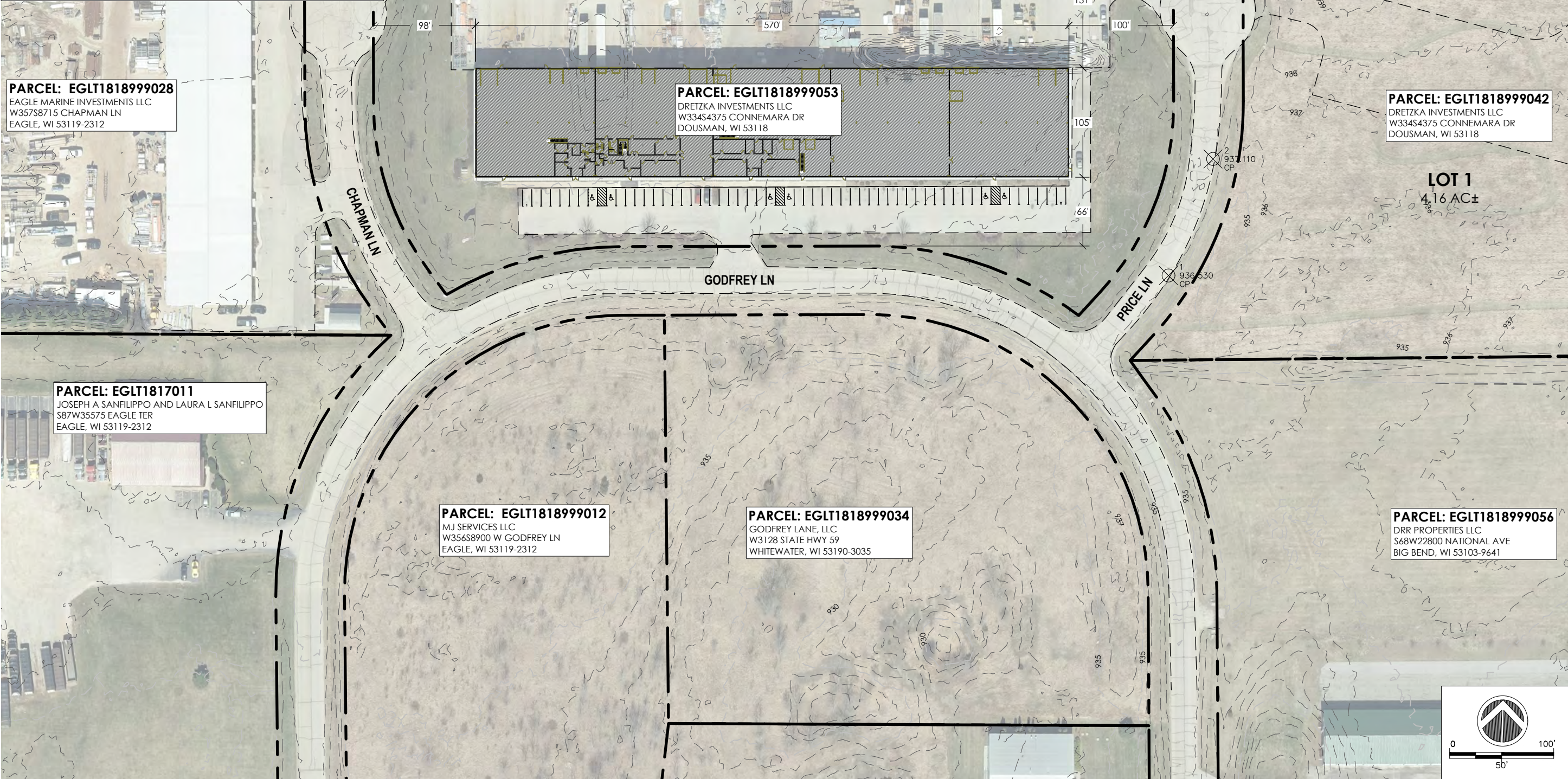
SITE ADDRESS: S87W35670 GODFREY LN, EAGLE, WI
PARCEL ID 1818999053
LOT 1 CERT SURV 10963 VOL 107/49 REC IN DOC# 3909481 PT NW1/4 SEC 23
T5N R17E :: DOC# 3420791
LOT AREA: 5.433 AC±

CURRENT ZONING: B-4 MIXED BUSINESS

BLDG SETBACKS:
NORTH = 40'
SOUTH = 66'
EAST = 100'
WEST = 98'

PROPOSED ADDITION: UFI MANUFACTURING FACILITY
EX. BUILDING FOOTPRINT = 59,850 SF (1.37± AC)
DRIVEWAY & PARKING = 106,786 SF (2.45± AC)
TOTAL IMPERVIOUS = 166,636 SF (3.83 AC) 70%
LANDSCAPE AREAS = 70,025 SF (1.60 AC) 30%

EXISTING PARKING STALLS = 55 STALLS / (3) ADA



| BENCHMARK TABLE | | |
|-----------------|--------------------|-----------|
| BENCHMARK | DESCRIPTION | ELEVATION |
| CP1 | PK NAIL IN ASPHALT | 936.53' |
| CP2 | PK NAIL IN ASPHALT | 937.11' |

PROJECT:

2025 BUILDING ADDITION

UFI MANUFACTURING

ADDRESS:

S87W35670
GODFREY LN, EAGLE
WI 53119

SHEET NAME:

EXISTING PLAN

REVISIONS

NO.

DATE

RES

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Verona, WI 53593

ADVANCED
BUILDING
CORPORATION

3802 KIPP ST.
MADISON, WI 53718

PROJECT NO: 0924-25

PLOT DATE: 11/27/25

PLOT BY: SSR

PLOT SCALE:

SHEET #

C1.0

BID DOCUMENTS

FILE NAME : \\ssr\res\Projects\2025\0924-25 UFI CAD\Sheets\Plan\C1.0.dwg

SITE DEVELOPMENT DATA:

SITE ADDRESS: S87W35670 GODFREY LN, EAGLE, WI
PARCEL ID 1818999053
LOT 1 CERT SURV 10963 VOL 107/49 REC IN DOC# 3909481 PT NW1/4 SEC 23
T5N R17E :: DOC# 3420791
LOT AREA: 5.433 AC±

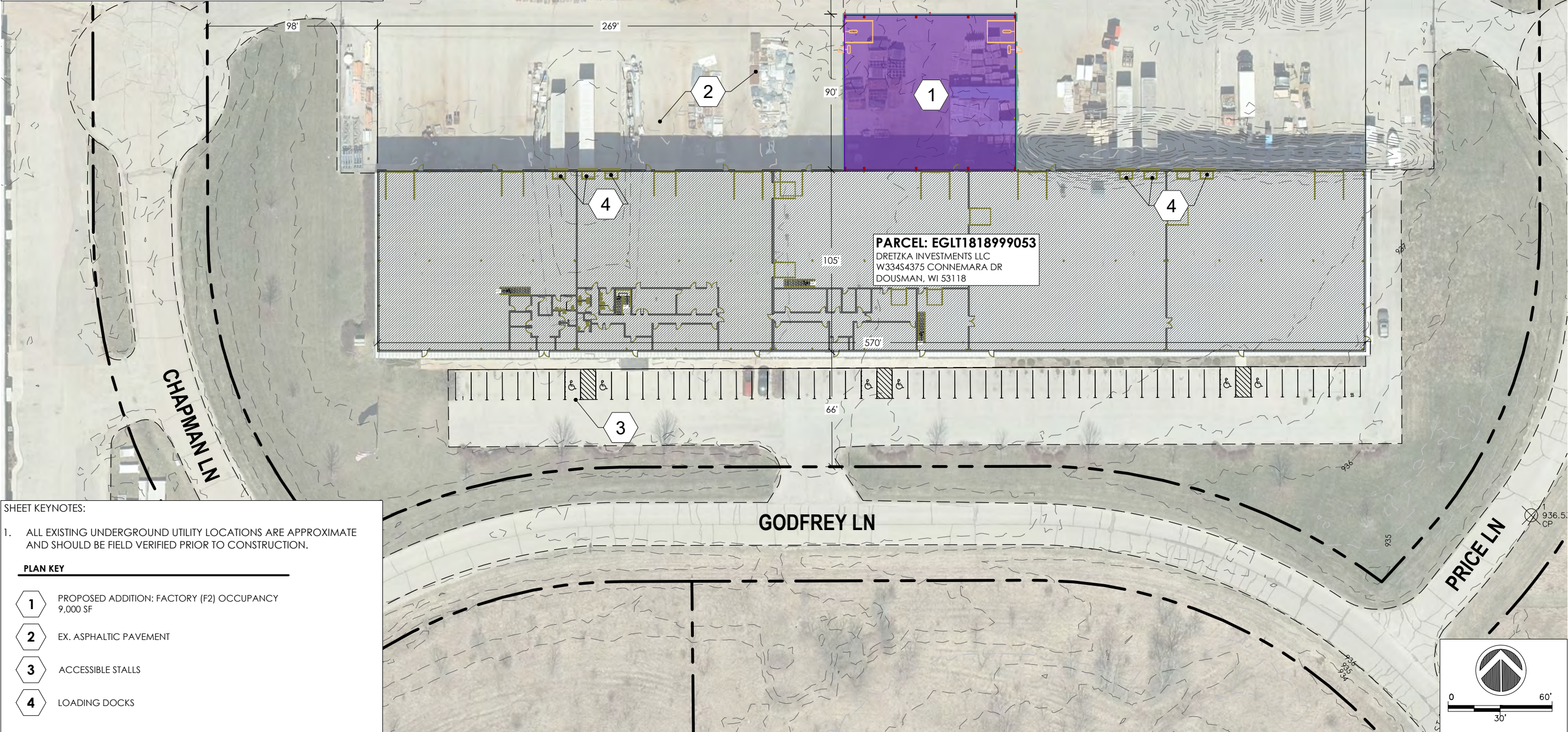
CURRENT ZONING: B-4 MIXED BUSINESS

BLDG SETBACKS:
NORTH = 40'
SOUTH = 66'
EAST = 100'
WEST = 98'

PROPOSED ADDITION: UFI MANUFACTURING FACILITY
EX. BUILDING FOOTPRINT = 68,885 SF (1.58± AC)
DRIVEWAY & PARKING = 97,751 SF (2.25± AC)
TOTAL IMPERVIOUS = 166,636 SF (3.83 AC) 70%
LANDSCAPE AREAS = 70,025 SF (1.60 AC) 30%

PROPOSED ADDITION = 9,000 SF (0.21± AC)
PROPOSED ADDITION REPLACES EXISTING PAVEMENT, THUS DOES NOT INCREASE IMPERVIOUS AREA.

EXISTING PARKING STALLS = 55 STALLS / (3) ADA





FILE NAME : \\ssr\res\Projects\2025\0924-25 UPI\CAD\Sheets\Plan\C3.0.dwg


EROSION CONTROL NOTES:

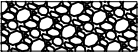
1. THE CONTRACTOR SHALL IMPLEMENT AND MAINTAIN EROSION CONTROL DEVICES FROM THE START OF LAND DISTURBING CONSTRUCTION ACTIVITIES UNTIL FINAL STABILIZATION OF THE CONSTRUCTION SITE.
2. ALL EROSION CONTROL MEASURES SHALL BE INSTALLED IN ACCORDANCE WITH TOWN EROSION CONTROL AND STORMWATER ORDINANCE AND THE APPROPRIATE WISCONSIN DNR CONSTRUCTION SITE EROSION AND SEDIMENT CONTROL TECHNICAL STANDARDS. THE TOWN RESERVES THE RIGHT TO REQUIRE ADDITIONAL EROSION CONTROL MEASURES AS CONDITIONS WARRANT.
3. CLEANING RIGHT-OF-WAY SURFACES SHALL BE THOROUGHLY CLEANED BEFORE THE END OF EACH WORKING DAY WITHOUT HYDRAULIC FLUSHING.
4. RESTORATION RESTORATION SHALL BE COMPLETED AS NOTED IN THE CONSTRUCTION SCHEDULE UNLESS OTHERWISE AUTHORIZED BY THE TOWN.
5. INSPECTION THE CONTRACTOR SHALL INSPECT EROSION AND SEDIMENT CONTROL PRACTICES WEEKLY, AND WITHIN 24 HOURS FOLLOWING A RAINFALL OF 0.5 INCHES OR GREATER. DOCUMENTATION OF EACH INSPECTION SHALL INCLUDE THE TIME, DATE AND LOCATION OF INSPECTION, THE PHASE OF LAND DISTURBANCE AT THE CONSTRUCTION SITE, PERSON CONDUCTING THE INSPECTION, ASSESSMENT OF CONTROL PRACTICES, AND A DESCRIPTION OF ANY EROSION OR SEDIMENT CONTROL MEASURE INSTALLATION.
6. STONE TRACKING PAD. BEFORE BEGINNING CONSTRUCTION, CONTRACTOR SHALL INSTALL A STONE TRACKING PAD AT EACH POINT WHERE VEHICLES ENTERS/EXITS THE CONSTRUCTION SITE. STONE TRACKING PADS SHOULD BE AT LEAST 24 FEET WIDE AND 50 FEET LONG, AND CONSTRUCTED OF 3-6 INCH WASHED STONE WITH A DEPTH OF AT LEAST 12 INCHES. ON SITES WITH CLAY SOILS, TONE TRACKING PADS MUST BE UNDERLAIN WITH A GEOTEXTILE LINER TO PREVENT THE STONE FROM SINKING INTO THE SOIL.
7. INLET PROTECTION. ALL INLETS SUBJECT TO DRAINAGE SHALL BE PROTECTED WITH TYPE D INLET PROTECTION OR APPROVED EQUAL. ANY DEPOSITS OF DIRT, MUD, ROCK, DEBRIS, OR OTHER MATERIAL ENTERING THE STORM SEWER SYSTEM SHALL BE PROMPTLY AND THOROUGHLY CLEANED OUT.
8. EROSION MAT. AREAS OR EMBANKMENTS HAVING SLOPES GREATER THAN OR EQUAL TO 4H:1V SHALL BE STABILIZED WITH WISCONSIN DOT CLASS 1, URBAN, TYPE B, EROSION MAT.
9. TEMPORARY EROSION CONTROL. ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES SHALL BE REMOVED WITHIN 30 DAYS AFTER FINAL SITE STABILIZATION IS ACHIEVED OR AFTER THE TEMPORARY MEASURES ARE NO LONGER NEEDED.
10. STABILIZATION. STABILIZE AREAS OF FINAL GRADING WITHIN 7 DAYS OF REACHING FINAL GRADE.
11. TOPSOIL. A MINIMUM OF 4 INCHES OF TOPSOIL MUST BE APPLIED TO ALL AREAS TO BE SEEDED OR SODDED.
12. SEEDING. SEED MIXTURE SHALL BE APPLIED AT A RATE OF 2 LBS PER 1,000 SF OF AREA OR AS NOTED ON THE LANDSCAPE PLAN.
13. STOCKPILES. IMMEDIATELY STABILIZE STOCKPILES AND SURROUND STOCKPILES AS NEEDED WITH SILT FENCE OR OTHER PERIMETER CONTROL IF STOCKPILES WILL REMAIN INACTIVE FOR 7 DAYS OR LONGER.
14. DEWATERING SHALL FOLLOW DNR TECHNICAL STANDARD 1061.
15. CONCRETE WASHOUT AREA SHALL BE DESIGNATED BMP PER DNR TECHNICAL STANDARD.


EROSION CONTROL KEY


- 

TRACKING PAD
- 

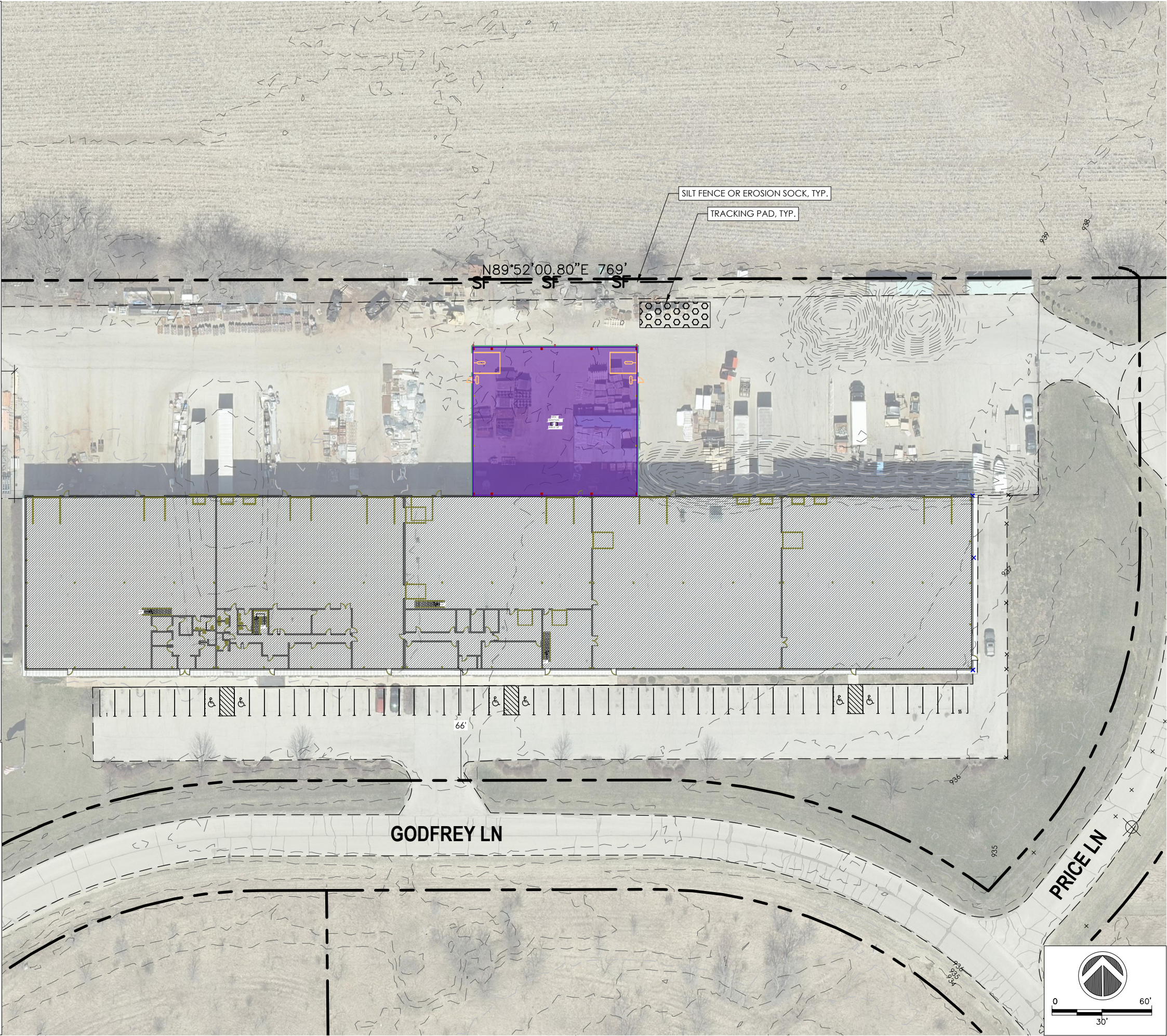
SILT FENCE, TYP.
- 

INLET PROTECTION, TYP.
- 

CLASS II RIP-RAP @ STORM SEWER OUTLET, TYP.
- 

EROSION MAT
- 

TEMPORARY DITCH CHECK, TYP.



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Verona, WI 53593



3802 KIPP ST.
MADISON, WI 53718

PROJECT NO: 0924-25
PLOT DATE: 12/2/25
PLOT BY: SSR
PLOT SCALE:

SHEET # C3.0

BID DOCUMENTS

PROJECT:
2025 BUILDING ADDITION
UPI MANUFACTURING

ADDRESS:
S87W35670
GODFREY LN, EAGLE
WI 53119

SHEET NAME:
EROSION CONTROL PLAN

REVISIONS
NO. DATE

③



2-TRACKING PAD

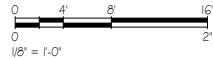
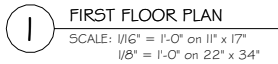


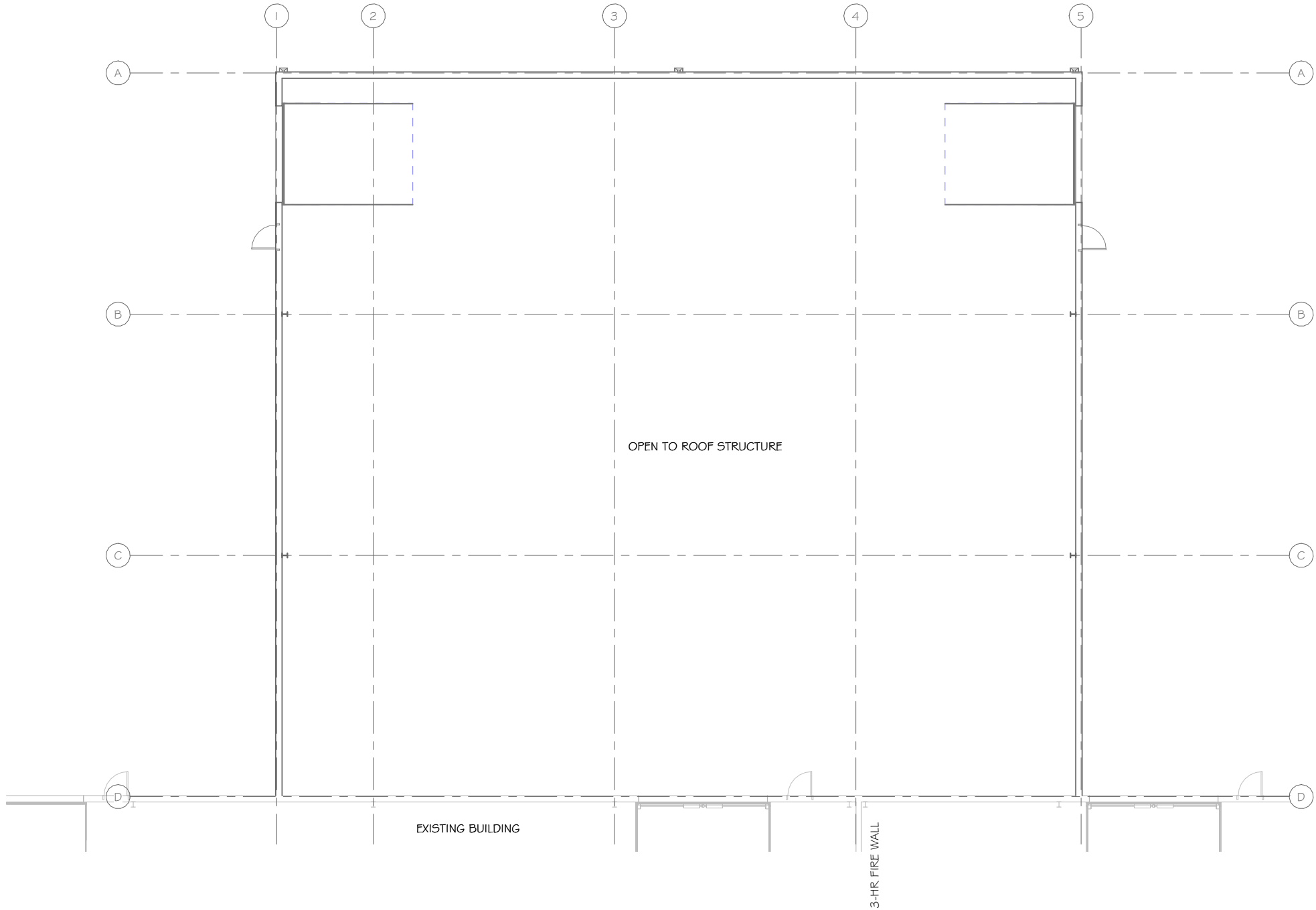
3-TEMPORARY DITCH CHECK

Verona, WI 53593 608-620-3036 (Office) • svet@roussevingeering.com

Verona, WI 53593 60


BID DOCUMENTS






1 FIRST FLR. REFLECTED CEILING PLAN
SCALE: 1/16" = 1'-0" on 11" x 17"
1/8" = 1'-0" on 22" x 34"

| CEILING LEGEND | |
|----------------|---------------------------------------------|
| | 24" X 24" ACOUSTICAL CEILING TILE |
| | GYPSUM WALL BOARD / PAINT |
| | EXPOSED CEILING TO ROOF DECK |
| | FLUORESCENT STRIP FIXTURE |
| | RECESSED FLUORESCENT FIXTURE |
| | LIGHTING FIXTURES ON EMERGENCY POWER SOURCE |
| | SUPPLY FRESH AIR |
| | RETURN AIR REGISTER |
| | CEILING MOUNTED OCCUPANCY SENSOR |
| | CARBON MONOXIDE DETECTORS |
| | SPRINKLER HEADRE |
| | EXIT LIGHT FIXTURE |



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7563 Driftless Rdg Wy
Verona, WI 53593



3802 KIPP ST.
MADISON, WI 53718

PROJECT:
2025 BUILDING ADDITION
UPI MANUFACTURING

ADDRESS:
S87W35G7O
GODFREY LN
EAGLE, WI 53119

SHEET NAME:
REFLECTED CEILING
PLAN

REVISIONS
NO. DATE

PROJECT NO:
PLOT DATE:
PLOT BY:
PLOT SCALE:

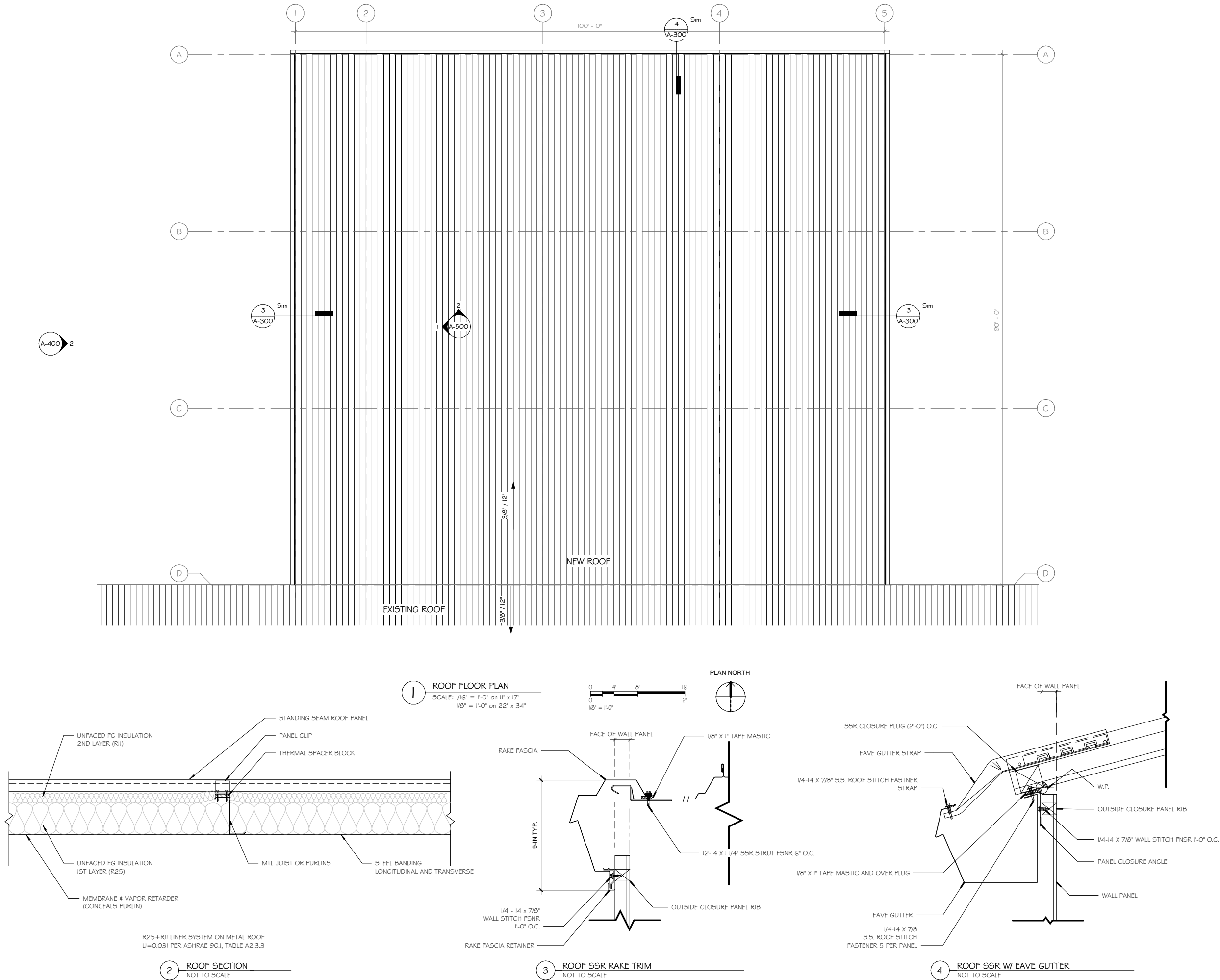
0924-25
11/1/25
SSR
1/8" = 1'-0"

SHEET #

A-200

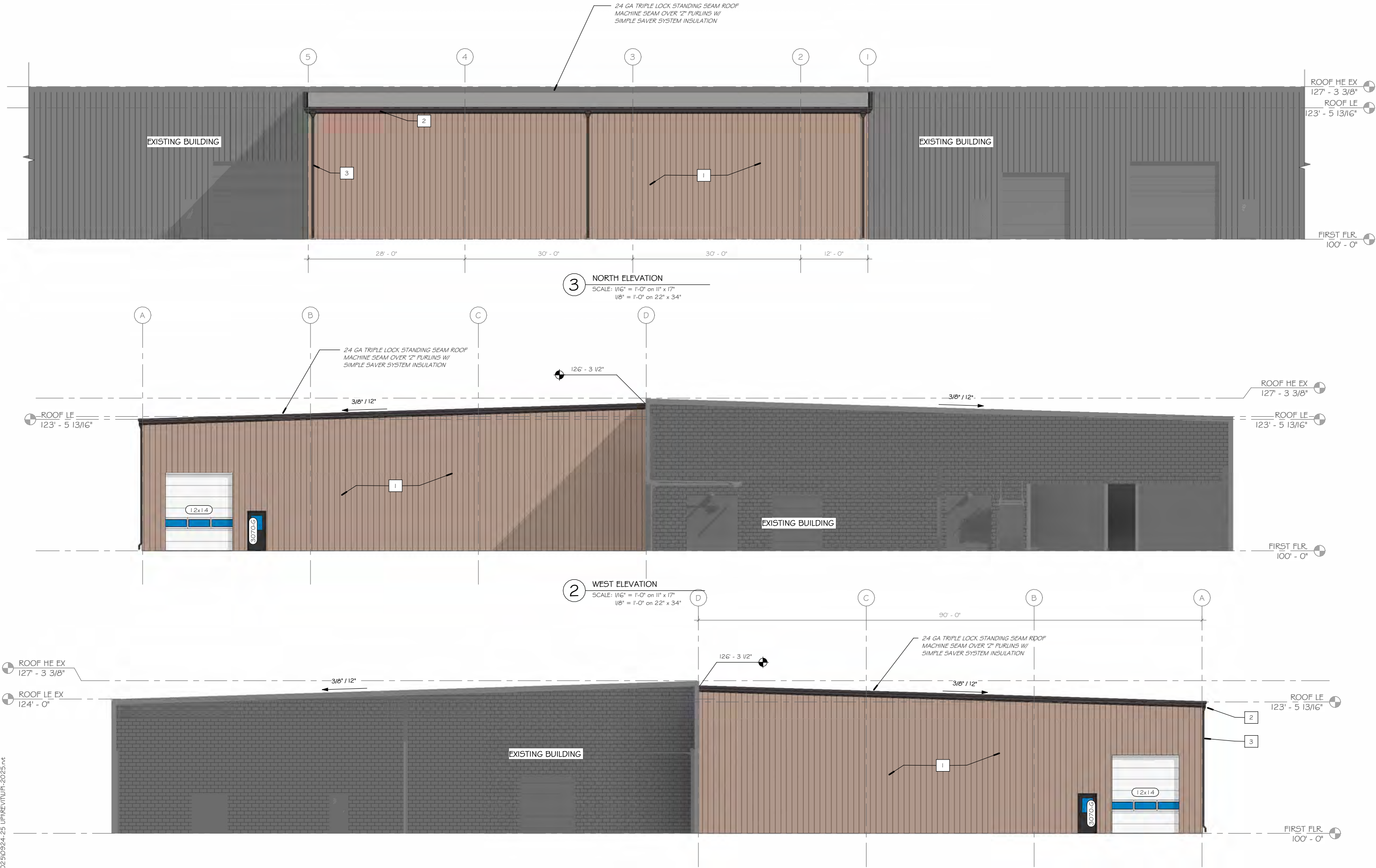
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E:\Projects\2025\0924-25 UPI REV\TUP-2025.rvt

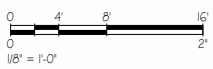


| PROJECT: | ADDRESS: | SHEET NAME: | REVISIONS | NO. | DATE |
|-------------------------------------------------------------------------------------------------------------------------------------------------------------|---------------------------------------------|-------------|-----------|-----|------|
| | | | | | |
| RES ROUSSEV ENGINEERING SOLUTIONS, LLC 7563 Driftless Rdg Wy Verona, WI 53593 608-620-3036 (Office) • svet@rousseveengineering.com | 2025 BUILDING ADDITION UPI MANUFACTURING | ROOF PLAN | | | |
| | | | | | |
| PROJECT NO: 0924-25 | | | | | |
| PLOT DATE: 11/1/25 | | | | | |
| PLOT BY: SSR | | | | | |
| PLOT SCALE: As indicated | | | | | |
| SHEET # | A-300 | | | | |
| BID DOCUMENTS | | | | | |


FILE NAME: E:\Projects\2025\0924-25 UPI REV\TUP-2025.rvt



1 EAST ELEVATION
SCALE: 1/8" = 1'-0" on 11" x 17"
1/4" = 1'-0" on 22" x 34"




| ELEVATION NOTES | |
|-----------------|-----------------------------------------------------------|
| Item | Description |
| 1 | STEEL - PANEL RIB - AMERICAN BUILDING SURREY BEIGE |
| 2 | 6"x6" GUTTER: COLOR AMERICAN BUILDINGS -MIDNIGHT BLACK |
| 3 | 5"x6" DOWNSPOUT: COLOR AMERICAN BUILDINGS -MIDNIGHT BLACK |



ROUSSEV
ENGINEERING
SOLUTIONS, LLC

PROJECT:

2025 BUILDING ADDITION
UPI MANUFACTURING



3802 KIPP ST.
MADISON, WI 53718

PROJECT NO:

0924-25

PLOT DATE:

11/1/25

PLOT BY:

SSR

PLOT SCALE:

1/8" = 1'-0"

SHEET #

A-400

BID DOCUMENTS

SHEET NAME:

EXTERIOR ELEVATIONS

ADDRESS:

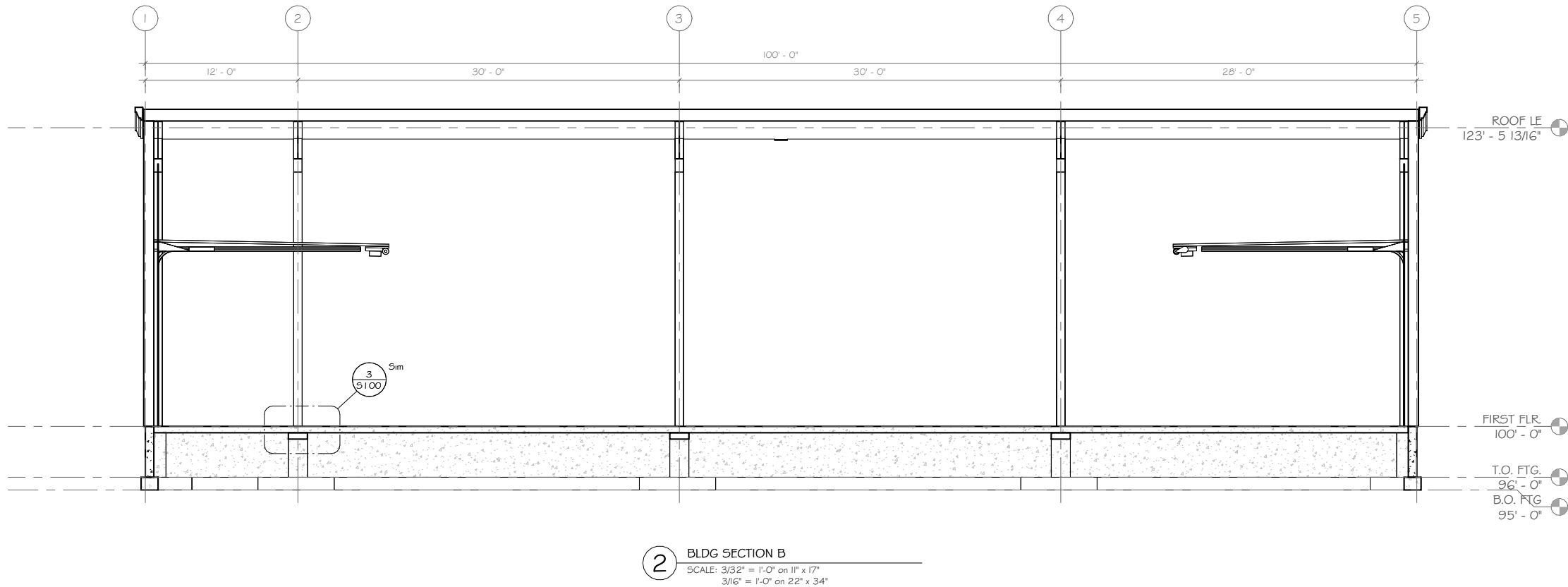
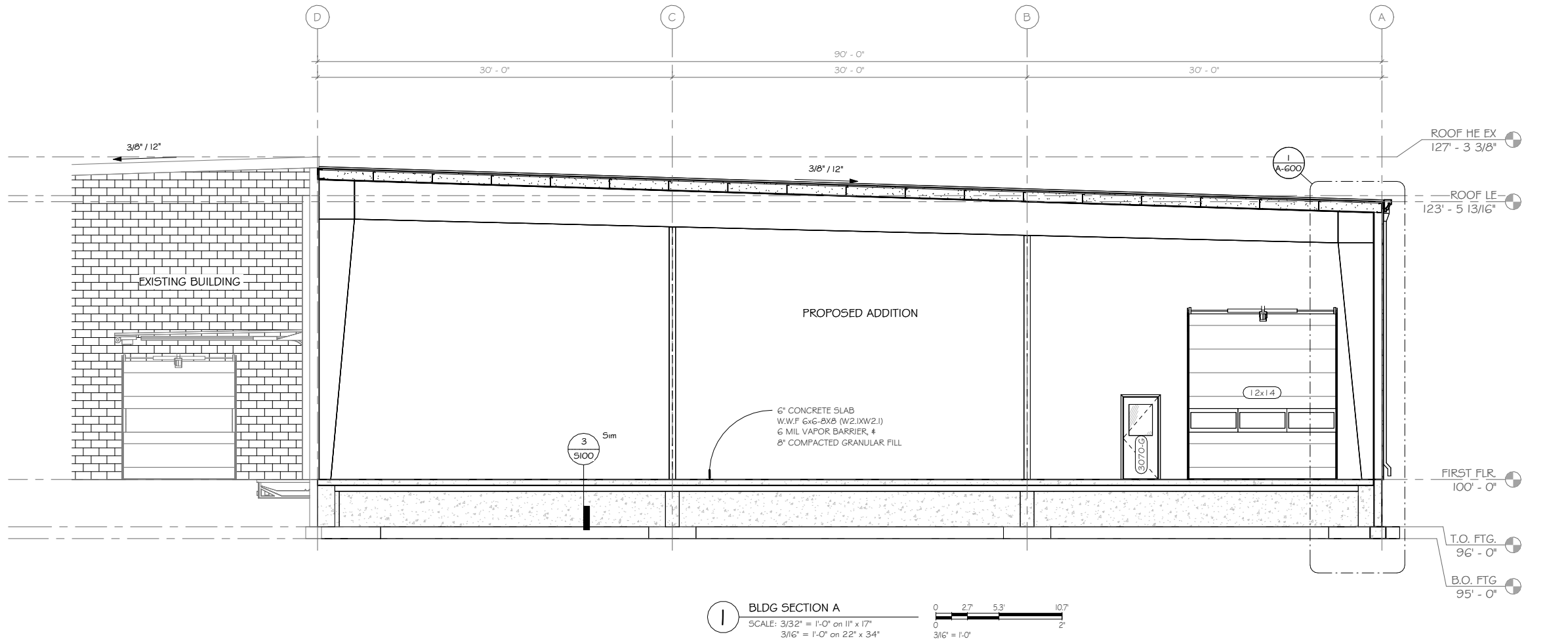
S87W35C70
GODFREY LN
EAGLE, WI 53119

NO.

DATE

REVISIONS

FILE NAME : E:\Projects\2025\0924-25 UPI REV\TUP-2025.rvt



PROJECT NO: 0924-25
PLOT DATE: 11/1/25
PLOT BY: SSR
PLOT SCALE: 3/16" = 1'-0"

SHEET # A-500

BID DOCUMENTS

PROJECT:

2025 BUILDING ADDITION
UPI MANUFACTURING

ROUSSEV
ENGINEERING
SOLUTIONS, LLC
7563 Driftless Rdg Wy
Verona, WI 53593

608-620-3036 (Office) • svet@roussevengeenring.com

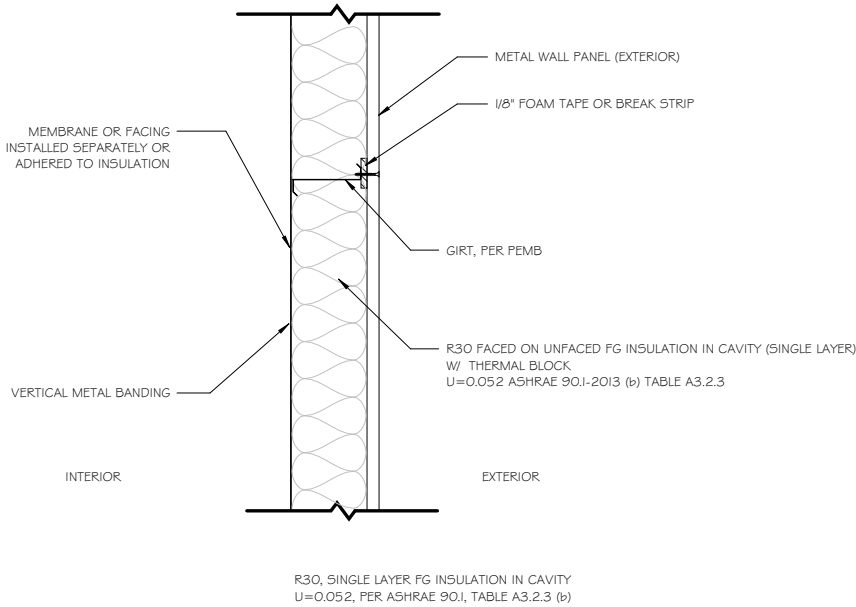
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S87W35670
GODFREY LN
EAGLE, WI 53119

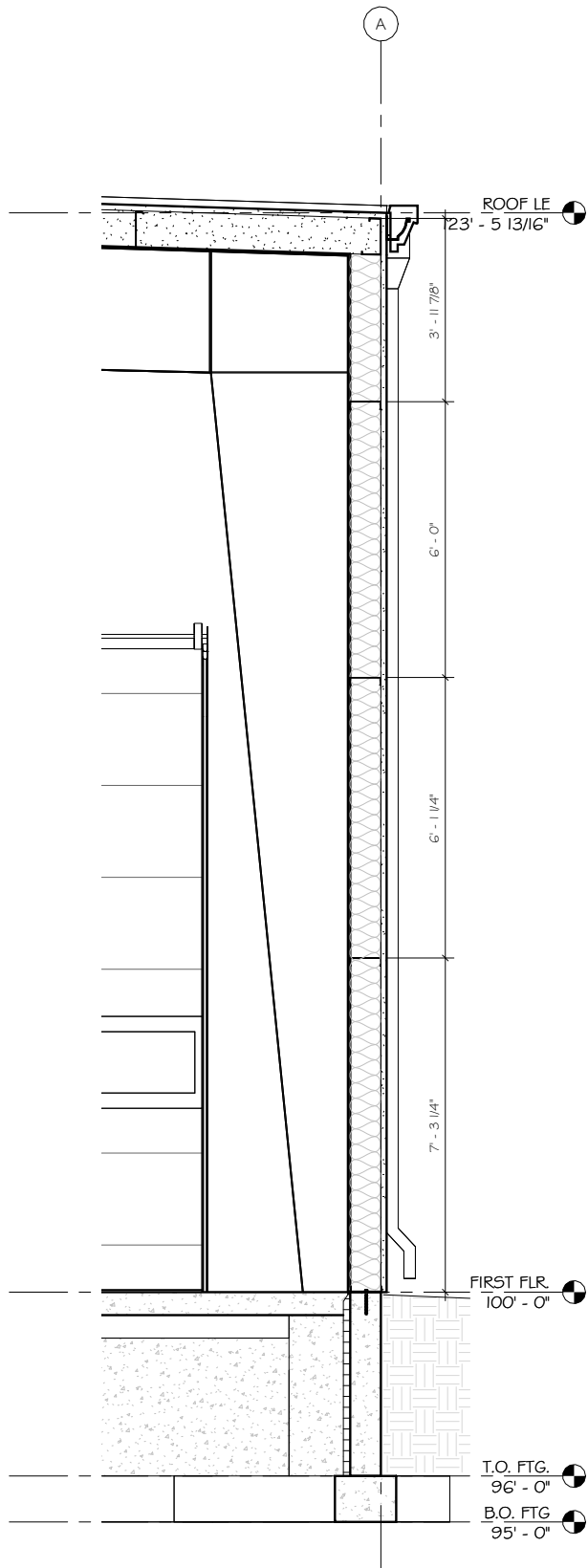
SHEET NAME:
BUILDING SECTIONS

REVISIONS

NO. DATE



2 EXTERIOR WALL SECTION
NOT TO SCALE



1 WALL DETAIL
SCALE: 1/4" = 1'-0" on 11" x 17"
1/2" = 1'-0" on 22" x 34"



3802 KIPP ST.
MADISON, WI 53718

PROJECT NO: 0924-25
PLOT DATE: 11/11/25
PLOT BY: SSR
PLOT SCALE: As indicated

SHEET # A-600

BID DOCUMENTS

PROJECT:

2025 BUILDING ADDITION
UPI MANUFACTURING

ROUSSEV
ENGINEERING
SOLUTIONS, LLC
7563 Driftless Rdg Wy
Verona, WI 53593



ADDRESS:
S87W35670
GODFREY LN
EAGLE, WI 53119

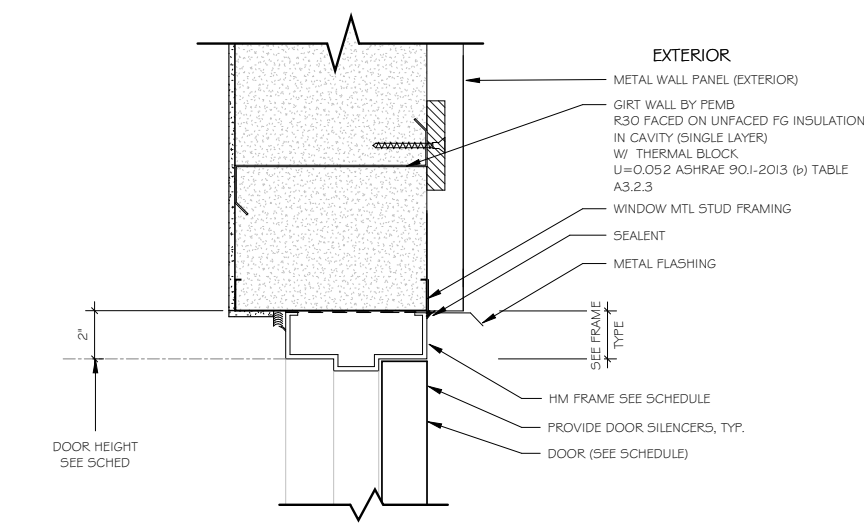
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WALL SECTIONS

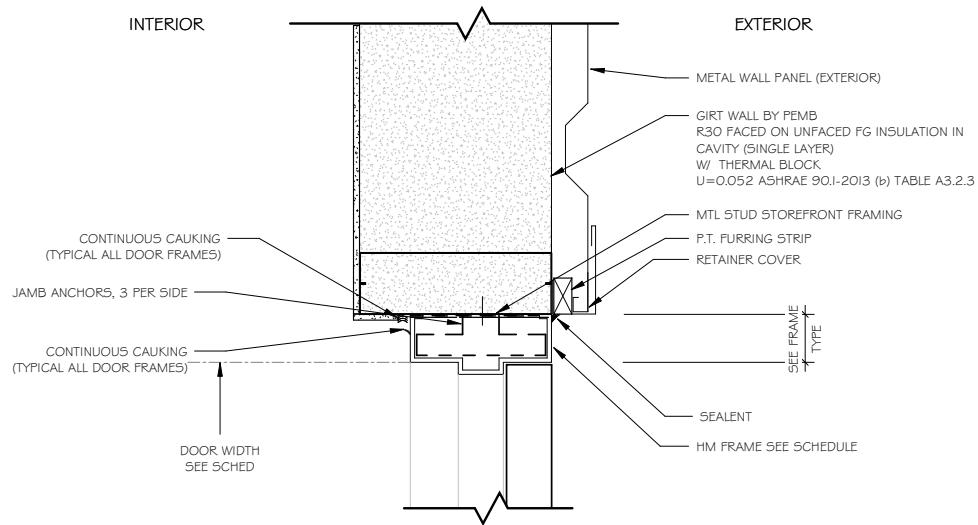
REVISIONS

NO. DATE

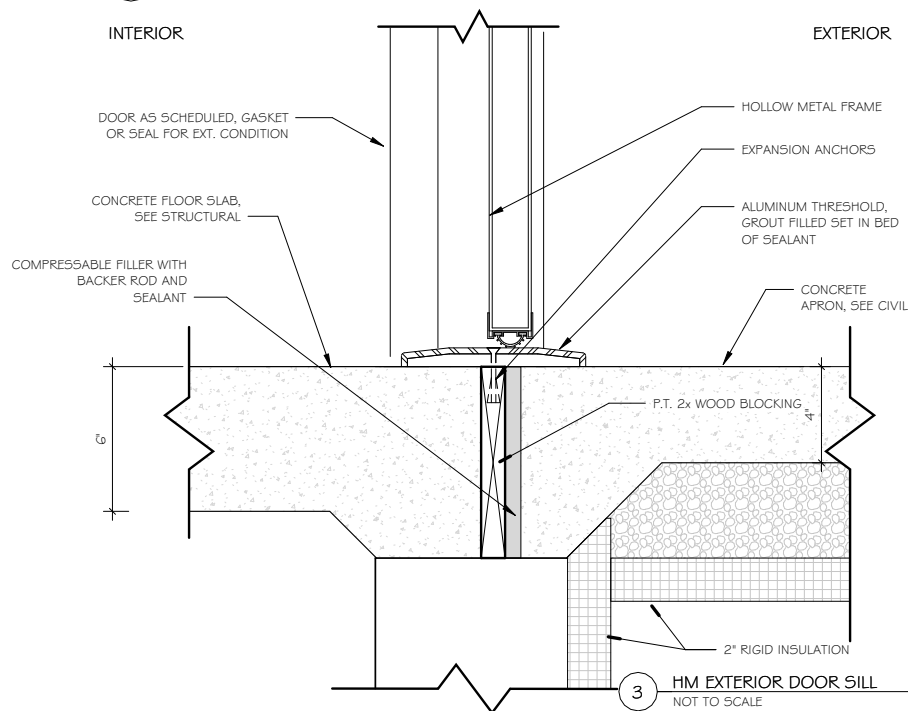
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2 HM EXTERIOR DOOR HEAD
NOT TO SCALE

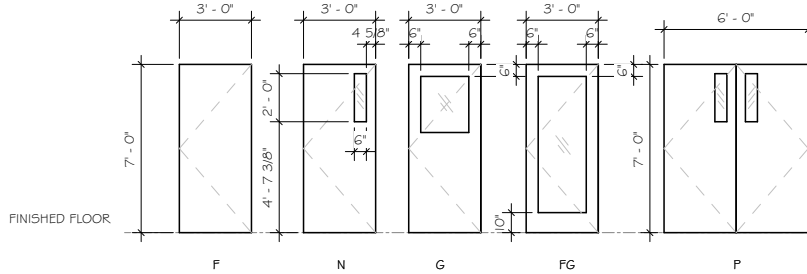


1 HM EXTERIOR DOOR JAMB
NOT TO SCALE

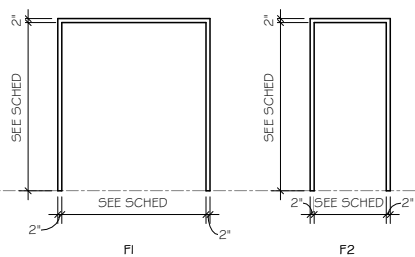


3 HM EXTERIOR DOOR SILL
NOT TO SCALE

DOOR TYPES:



FRAME TYPES:



| DOOR SCHEDULE | | | | | | | | | |
|----------------|----------|----------|--------|------|-------|------|-----|-------|---------------|
| Mark | SIZE | | | DOOR | FRAME | | GLZ | NOTES | |
| | W | HT | THK | | MAT'L | ELEV | | | |
| I00 | 3' - 0" | 7' - 0" | 1 3/4" | G | HM | F2 | HM | GL-1 | |
| I01 | 3' - 0" | 7' - 0" | 1 3/4" | G | HM | G2 | HM | GL-1 | |
| I02 | 12' - 0" | 14' - 0" | 1 3/4" | | | | | | OVERHEAD DOOR |
| I03 | 12' - 0" | 14' - 0" | 1 3/4" | | | | | | OVERHEAD DOOR |
| Grand total: 4 | | | | | | | | | |

GENERAL NOTES - DOORS

- ALL DOORS SHALL HAVE A MINIMUM STC RATING OF 35 UNLESS NOTED OTHERWISE.
- ALL EXTERIOR DOORS NOT LOCATED IN STOREFRONT / CURTAINWALL ASSEMBLIES SHALL BE INSULATED HOLLOW METAL AND MINIMUM LEVEL 2 PERFORMANCE LEVEL B PER SD/DOOR A250.8. ALL DOOR FRAMES SHALL BE WELDED. DOOR FRAMES SHALL BE FILLED WITH SPRAY FOAM INSULATION. EXTERIOR DOORS SHALL EXCEED MINIMUM INSULATION REQUIRED IN A SHRAE 90.1 - IP.
- INTERIOR VESTIBULE DOORS SHALL MATCH THE TYPE, CONSTRUCTION AND CONFIGURATION OF THEIR CORRESPONDING EXTERIOR DOORS.
- INTERIOR DOORS SHALL BE SOLID CORE WOOD VENEER DOOR. FRAMES SHALL BE WELDED HOLLOW METAL. HM DOORS AND FRAMES SHALL RECEIVE A PAINTED FINISH.
- LOCKSETS MUST ACCEPT MARSHALL BEST 6-PIN REMOVABLE CORE.
- THERMALLY BROKEN FRAME
- INSULATED DOOR
- TRACK & HARDWARE BY MANUFACTURER
- UL RATED DOOR & FRAME ASSEMBLY
- UNDERCUT DOOR IF
- PROVIDE ALL EXTERIOR EGRESS DOORS WITH PANIC HARDWARE OR FIRE EXIT HARDWARE PER IBC 1010.1.10. MAIN EXIT DOORS ARE PERMITTED TO BE LOCKING PER IBC 1010.1.9.3

LEGEND - DOOR TYPES

- F FLUSH
- P PAIR DOOR
- L LOUVER (BOTTOM)
- TL LOUVER (TOP)
- LL LOUVER (TOP AND BOTTOM)
- V VISION LITE
- VL VISION LITE AND LOUVER
- N NARROW LITE
- NL NARROW LITE AND LOUVER
- G HALF GLASS
- GL HALF GLASS AND LOUVER
- FG FULL GLASS
- FL FULL LOUVER
- D DUTCH DOOR
- OHD OVERHEAD SECTIONAL DOOR
- OHC OVERHEAD COILING DOOR

DOOR ABBREVIATIONS

- ALUM ALUMINUM
- HM HOLLOW METAL
- MTL METAL
- PNT PAINTED
- INS INSULATED
- ANOD ANNOIDIZED
- FRP FIBRE REINFORCED POLYMER
- WD WOOD SOLID CORE

GLAZING SCHEDULE

- GL-1 EXTERIOR INSULATED GLAZING UNIT
- GL-2 FIRE RATED GLASS (D-H-60 OR D-H-W-60)
- GL-3 TEMPERED SAFETY GLASS

GLAZING SCHEDULE

- GL-1 EXTERIOR INSULATED & TEMPERED GLAZING UNIT
- GL-2 FIRE RATED GLASS (D-H-60 OR D-H-W-60)
- GL-3 TEMPERED SAFETY GLASS



ROUSSEV
ENGINEERING
SOLUTIONS, LLC
7563 Driftless Rdg Wy
Verona, WI 53593



3802 KIPP ST,
MADISON, WI 53718

PROJECT NO: 0924-25
PLOT DATE: 11/1/25
PLOT BY: SSR
PLOT SCALE: As indicated

SHEET # A-610

BID DOCUMENTS

SHEET NAME:

DOOR & WINDOW
SCHEDULES

ADDRESS:

S87W35670
GODFREY LN
EAGLE, WI 53119

PROJECT:

2025 BUILDING ADDITION
UPI MANUFACTURING

REVISIONS

NO. DATE

CONCRETE CAST-IN-PLACE NOTES:

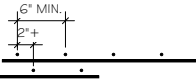
1. ALL CONCRETE DESIGN AND CONSTRUCTION SHALL CONFORM TO THE LOCAL BUILDING CODE REQUIREMENTS AND THOSE OF THE LATEST EDITION OF THE FOLLOWING STANDARDS: ACI 318, ACI 315, ACI 301, AND ACI 305 # 306.
2. CONCRETE SLABS ON GROUND CONTAINING REINFORCEMENT SHALL PLACE ALL REINFORCING BARS AND WMF ON CHAIRS, TIED IN PLACE, AND LOCATED IN THE MIDDLE TO THE UPPER ONE-THIRD OF THE SLAB. LIFTING REINFORCING AFTER CONCRETE IS PLACED IS NOT CONSIDERED TO BE AN EFFECTIVE MEANS OF PLACEMENT AND SHALL NOT BE ALLOWED WITHOUT PRIOR WRITTEN CONSENT OF THE ENGINEER. WELDED WIRE REINFORCEMENT FABRIC SHALL BE SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACING NOT TO EXCEED 3 FEET OR IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS. WELDED PLAIN WIRE REINFORCEMENT FABRIC FOR CONCRETE SHALL CONFORM TO ASTM A 185.
3. ALL CONCRETE MIX DESIGNS SHALL MINIMIZE SHRINKAGE AS MUCH AS IS PRACTICAL INCLUDING SELECTION OF AGGREGATE TYPE, SIZE, GRADATIONS W/ C RATIO AND ADD MIXTURES.
4. UNLESS THE MIX DESIGN INCLUDES THE USE OF SUPERPLASTICIZERS, CONCRETE WITH A SLUMP GREATER THAN 5" SHALL BE REFUSED.
5. ALL CONCRETE REINFORCING STEEL TO BE ASTM A615 GRADE 60. ALL WELDED WIRE FABRIC (WWF) TO BE ASTM A - 185. ALL REINFORCING SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315 AND 315R.
6. ALL REINFORCING BARS AND WMF SHALL BE SET ON CHAIRS AND TIED IN PLACE.
7. AFTER CONCRETING HAS STARTED, IT SHALL BE CARRIED ON AS A CONTINUOUS OPERATION UNTIL PLACING OF A PANEL OR SECTION, AS DEFINED BY ITS BOUNDARIES OR PREDETERMINED JOINTS, IS COMPLETED. CONCRETE SHALL BE DEPOSITED AS NEARLY AS PRACTICABLE TO ITS FINAL POSITION TO AVOID SEGREGATION DUE TO REHANDLING OR FLOWING.
8. CONCRETING OPERATIONS SHALL BE CARRIED ON AT SUCH A RATE THAT THE CONCRETE IS AT ALL TIMES PLASTIC AND FLOWS READILY INTO SPACES BETWEEN REINFORCEMENT.
9. CONCRETE SHALL BE THOROUGHLY CONSOLIDATED BY SUITABLE MEANS DURING PLACEMENT AND SHALL BE THOROUGHLY WORKED AROUND REINFORCEMENT AND EMBEDDED FIXTURES AND INTO CORNERS OF THE FORMS. THE TOP SURFACES OF VERTICALLY FORMED LIFTS SHALL BE GENERALLY LEVEL.
10. CONCRETE SHALL BE CURED ABOVE 50°F (10°C) AND IN A MOIST CONDITION FOR AT LEAST THE FIRST SEVEN DAYS AFTER PLACEMENT. DO NOT PLACE CONCRETE WHEN DURING ANY POINT IN THE DAY THE MEAN DAYLIGHT TEMPERATURE IS LESS THAN 20°F.
11. ALL FLAT WORK CONCRETE SHALL BE COVERED IMMEDIATELY FOLLOWING SAW CUTTING AND MAINTAINED CONTINUOUSLY WET FOR A MINIMUM OF 7-DAYS AFTER PLACING. CURING SHEETS ARE TO BE USED AND REMAIN IN PLACE. CURING COMPOUNDS MAY BE USED APPLIED MUST BE APPLIED PER THE MANUFACTURER'S RECOMMENDATIONS. SUBMIT PRODUCT DATA TO A/E FOR APPROVAL.
12. RETEMPERED CONCRETE, CONCRETE THAT HAS BEEN REMIXED AFTER INITIAL SET OR PARTIALLY HARDENED SHALL NOT BE USED IN THE STRUCTURE.
13. ALL LAPS SHALL BE "B" SPLICES UNLESS NOTED OTHERWISE ON THE DRAWINGS OR UNLESS SPECIAL CARE IS TAKEN FOR THE REINFORCING TO BE DETAILED AND PLACED TO PROVIDE STAGGERED LAPS.
14. UNLESS OTHERWISE APPROVED, ALL EXPOSED CONCRETE WALLS SHALL BE CURED WITH FORMS LEFT IN PLACE FOR SEVEN DAYS. IF FORMS CAN NOT BE LEFT IN PLACE THE CONTRACTOR SHALL SUBMIT IN WRITING TO THE ENGINEER ALL PROPOSED CURING METHODS.
15. WALL CRACKS DUE TO IMPROPER CURING METHODS, OR WEATHER PROTECTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
16. ANCHOR BOLT DIAMETER AND PLACEMENT TO BE PER THE METAL BUILDING SUPPLIER'S DRAWINGS. ANCHOR RODS SHALL BE A MINIMUM OF (4) 3/4" DIAMETER F1554, GRADE 36, WITH A 9" MINIMUM EMBEDMENT UNLESS NOTED OTHERWISE. THREADED RODS SHALL HAVE A NUT AND WASHER SECURED TO THE EMBEDDED END EITHER BY WELD OR DOUBLE NUT.
17. THREADED RODS SHALL HAVE A NUT AND WASHER SECURED TO THE EMBEDDED END EITHER BY WELD OR DOUBLE NUT.
18. GROUT USED TO PROVIDE LEVEL BEARING OF COLUMN BASE PLATES SHALL BE NON-SHRINK, NON-METALLIC GROUT WITH A COMPRESSIVE STRENGTH 500 PSI OR MORE GREATER THAN THE COMPRESSIVE STRENGTH OF THE SUPPORTING CONCRETE MEMBER.
19. EPOXY FOR EPOXY GROUTED ANCHORS SHALL BE A TWO PART 100% SOLID EPOXY SUPPLIED AND DISPENSED THOUGH A STATIC MIXING NOZZLE SUPPLIED BY THE MANUFACTURE. DRILLED HOLES MUST BE BRUSHED CLEAN AND BLOWN OUT PRIOR TO INSTALLATION OF THE ANCHORS. FOLLOW ALL SUPPLIER'S INSTRUCTIONS FOR INSTALLATION.
20. UNLESS NOTED OTHERWISE ON THE DRAWINGS ALL REINFORCING SHALL BE LAPPED TO DEVELOP ITS CAPACITY AS FOLLOWS:

| UNCOATED STANDARD # CLASS "B" TENSION LAP LENGTHS | | | | | | | | | |
|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|----------|----------|----------------------|----------|-------------------------------------------|----------|----------|----------------------|----------|
| CONCRETE COMPRESSIVE STRENGTH = 3,500 PSI | | | | | CONCRETE COMPRESSIVE STRENGTH = 4,000 PSI | | | | |
| STD DEVELOPMENT LENGTH | | | CLASS "B" LAP LENGHT | | STD DEVELOPMENT LENGTH | | | CLASS "B" LAP LENGHT | |
| BAR SIZE | BTM BARS | TOP BARS | BTM BARS | TOP BARS | BAR SIZE | BTM BARS | TOP BARS | BTM BARS | TOP BARS |
| #3 | 15" | 20" | 20" | 26" | #3 | 14" | 18" | 18" | 23" |
| #4 | 20" | 30" | 30" | 39" | #4 | 19" | 25" | 25" | 33" |
| #5 | 25" | 38" | 38" | 50" | #5 | 24" | 31" | 31" | 40" |
| #6 | 30" | 46" | 46" | 60" | #6 | 28" | 37" | 37" | 48" |
| #7 | 35" | 53" | 53" | 69" | #7 | 33" | 43" | 43" | 56" |
| #8 | 41" | 61" | 61" | 79" | #8 | 38" | 49" | 49" | 64" |
| NOTES: 1. BASED ON "d" VALUES FROM SECTION 25.4.2.2 IN ACI 318-14: 1.1 GRADE 60 REINFORCEMENT BARS 1.2 NORMAL WEIGHT CONCRETE (=1.0) 1.3 NON-EPOXY COATED BARS (=1.0) 1.4 CLEAR COVER > 1.0d; CLEAR SPACING > 2.0d; NOTIFY ENGINEER IF COVER NOT MET. 2. STANDARD LAB SPLICES ARE TO BE USED WHEN < 50% OF BARS ARE LAPPED AT THE SAME LOCATION, INCLUDING TEMPERATURE AND SHRINKAGE LAP SPLICES. 2.1 PROVIDE CLASS B LAP SPLICES UNLESS DETAILED AND APPROVED BY ENGINEER. 3. CLASS B LAP SPLICES ARE TO BE USED WHEN > 50% OF BARS ARE LAPPED AT THE SAME LOCATION, INCLUDING TEMPERATURE AND SHRINKAGE LAP SPLICES. 4. HORIZONTAL (TOP) BARS HAVE MORE THAN 12" OF FRESH CONCRETE PLACED BELOW REINFORCEMENT. SPLICE LENGTHS ARE ROUNDED UP TO THE NEAREST INCH. | | | | | | | | | |

21. SLAB ON GRADE SHALL HAVE A CLASS "A" TOLERANCE.
22. A 10-MIL (MIN.) POLYETHYLENE VAPOR BARRIER WITH JOINTS LAPPED NOT LESS THAN 6" SHALL BE PLACED BETWEEN THE BASE COURSE OR SUBGRADE AND THE CONCRETE FLOOR.
23. CALCIUM CHLORIDE AND OR ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL NOT BE USED.
24. PLACING OF CONCRETE SHALL BE DONE IN CONFORMANCE WITH ACI-306 FOR COLD WEATHER AND ACI-305 FOR HOT WEATHER.
25. EXPOSED FOUNDATION WALLS SHALL HAVE VERTICAL CONTROL JOINTS SPACED NOT MORE THAN 25'-0" ON CENTER. EACH JOINT SHALL BE 3/4" WIDE BY 1/4 WALL DEPTH DEEP AND V-CHAMFERED ON BOTH SIDES. HORIZONTAL WALL REINFORCING SHALL BE DISCONTINUOUS AT THE CONTROL JOINT LOCATION WITH GREASED SMOOTH DOVEL BARS AT 16" ON CENTER THRU THE JOINT. THE LOCATION OF WALL CONTROL JOINTS SHALL BE MID BAY BETWEEN COLUMNS.
26. EXPOSED FOUNDATION WALLS SHALL HAVE EXPANSION JOINTS LOCATED AT EVERY FOURTH CONTROL/CONTRACTION JOINT. SEE CONCRETE DETAILS FOR SPECIFIC CONSTRUCTION REQUIREMENTS.
27. FLOOR SLAB CONTROL JOINTS SHALL FOLLOW THE INTENT SHOWN ON THE PLAN BUT SHALL NOT EXCEED AN ASPECT RATIO OF 1.5 TO 1.0. ALL REINTRANT CORNERS SHALL HAVE CONTROL JOINTS EXTENDING OUT FROM THE INSIDE CORNER. DEAD-END "T" CONTROL JOINTS INTO CONTINUOUS JOINTS SHALL BE AVOIDED.
28. WALL EXPANSION JOINTS ARE REQUIRED WHERE INDICATED ON THE DRAWINGS BUT NOT TO EXCEED 125 FEET.
29. NO TACK WELDING WILL BE PERMITTED ON ASTM A615 GRADE 40 OR 60 STEEL.
30. CONTROL JOINTS SHALL BE CUT IN SLAB-ON-GRADE CONSTRUCTION WITHIN 24 HOURS OF INITIAL POUR.
31. CONSTRUCTION JOINTS SHALL BE LOCATED AT CONTROL JOINTS OR CONTRACTION JOINTS.
32. ALL SLAB-ON-GRADE SHALL UTILIZE 1.5 INCH TOP AGGREGATE IN AN EVENLY DISTRIBUTED AGGREGATE GRADATION.
33. PIPE SLEEVES OVER 1 1/2" IN DIAMETER WHICH PASS THROUGH CONCRETE WALLS OR SLABS SHALL BE SCHEDULE 40 GALVANIZED STEEL PIPE. ALL OTHER SLEEVES SHALL BE 18 GAUGE SHEET METAL. SLEEVES SHALL BE ON SIZE LARGER THAN OUTSIDE DIAMETER OF THE PIPE PASSING THROUGH THE SLEEVE. VERIFY SIZE AND NUMBER WITH MECHANICAL, ELECTRICAL, AND PLUMBING CONTRACTOR.
34. ALUMINUM CONDUIT SHALL NOT BE EMBEDDED IN CONCRETE.

CONCRETE REINFORCEMENT NOTES:

1. REINFORCING SHALL BE DETAILED IN ACCORDANCE WITH ACI 315 MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (MOST CURRENTLY ADOPTED EDITION).
2. PROVIDE MINIMUM COVER PER ACI 318, 7.7.1 ALSO SEE MILD STEEL PROTECTION NOTES.
3. WIRE SPACERS, CHAIRS, TIES, ETC. FOR SUPPORT OF STEEL REINFORCING SHALL BE PROVIDED BY THE CONCRETE CONTRACTOR TO ENSURE REINFORCING IS PLACED AND MAINTAINED IN THE PROPER POSITION DURING CONCRETE PLACEMENT.
4. ALL HOOKS IN STEEL REINFORCING SHALL BE ACI STANDARD HOOKS.
5. TERMINATE NON-CONTINUOUS STEEL REINFORCING WITH AN ACI STANDARD HOOK IF REQUIRED EMBEDMENT SHOWN ON DRAWINGS CANNOT BE OBTAINED.
6. ALL LAPS SHALL BE CLASS "B" PER ACI 318 ON THE DESIGN DRAWINGS, OR UNLESS THE DETAILER TAKES SPECIAL CARE TO PROVIDE STAGGERED LAPS. USE TO BAR LENGTHS FOR ALL HORIZONTAL WALL BARS AND FOR TOP BARS IN SLABS AND BEAMS OVER 12" DEEP.
7. STEEL REINFORCING SPLICES OF ADJACENT BARS SHALL BE STAGGERED SUCH THAT SPLICES ARE 4 FEET APART, MINIMUM.
8. CORNER BARS WITH CLASS "B" LAP PER ACI318 SHALL BE PROVIDED AT ALL WALL CORNERS AND AND INTERSECTIONS.
9. PROVIDE STEEL REINFORCING AROUND OPENINGS IN CONCRETE WALLS AND SLABS.
10. PROVIDE STEEL REINFORCING AT FOOTING STEPS.
11. WELDED WIRE REINFORCING SHALL BE IN FLAT SHEETS ONLY AND SHALL BE LAPPED AND/OR ANCHORED TO DEVELOP Fy PER ACI 315.



12. WELDING OF STEEL REINFORCEMENT IS NOT PERMITTED, UNLESS APPROVED BY ENGINEER.

MILD REINFORCING STEEL PROTECTION NOTES:

THE FOLLOWING MINIMUM DIMENSIONS SHALL BE PROVIDED AS A CLEAR COVER FOR REINFORCING BARS IN STRUCTURAL MEMBERS:

CONCRETE CAST AGAINST EARTH AND PERMANENTLY EXPOSED TO EARTH:

FOOTINGS 3"

CONCRETE PERMANENTLY EXPOSED TO EARTH OR WEATHER:

WALLS, COLUMNS, PIERS:

UP THROUGH #5 BARS 1-1/2"
#6 THROUGH #18 BARS 2"

CONCRETE NOT EXPOSED TO EARTH OR WEATHER:

WALLS: UP THROUGH #11 BARS 3/4"
#14 AND #18 BARS 1-1/2"

COLUMNS / PIERS: SIDES 1-1/2"

MISCELLANEOUS STRUCTURAL NOTES:

1. ENGINEER ASSUMES PIN BASED COLUMNS.
2. CONNECTORS:

A. FOR EXTERIOR AND INTERIOR APPLICATIONS WHERE EXPOSED TO MOISTURE, WHERE PRESSURE TREATED WOOD IS USED, AND FOR INTERIOR CORROSIVE ENVIRONMENTS ALL CONNECTORS SHALL BE HOT DIPPED GALVANIZED PER ASTM A 153A / 153M, OR STAINLESS STEEL, INCLUDING EXPANSION BOLTS, ANCHOR BOLTS, JOIST HANGERS, AND NAILS.

B. CONNECTION DESIGN TO WOOD OR STEEL FRAMING AND EVALUATION OF STRUCTURAL MEMBERS ADEQUACY BY A REGISTERED PROFESSIONAL ENGINEER SHALL BE PROVIDED BY ALL SUBCONTRACTORS.

C. INSTALLER OF ANCHORS OR CONNECTIONS TO STRUCTURE IS RESPONSIBLE FOR ANCHOR DESIGN AND DETERMINATION OF STRUCTURAL COMPONENT ADEQUACY. DO NOT CUT REINFORCING BARS OR DAMAGE OTHER EMBEDMENTS.
3. WORK BY OTHERS:

A. ALL SUPPORTS, FRAMING, SUB-FRAMING, LIGHT GAGE FRAMING, MISCELLANEOUS STEEL FRAMING, METAL FABRICATIONS, BRACING BRACKETS, HANGERS, CONNECTORS, EMBEDMENTS, FASTENERS, AND ATTACHMENTS NOT SHOWN ON THE STRUCTURAL DRAWINGS ARE THE CONTRACTOR'S RESPONSIBILITY AND SHALL BE ENGINEERED AND PROVIDED BY THE CONTRACTOR REQUIRING THE ITEM. COMPLY WITH GOVERNING CODES.

B. CONSTRUCTION MEANS AND METHODS ARE THE CONTRACTOR'S RESPONSIBILITY AND SHALL BE ENGINEERED AND PROVIDED BY THE CONTRACTOR REQUIRING SUCH, WORK INCLUDES, BUT IS NOT LIMITED TO:

1. EVALUATION OF STRUCTURE FOR CONSTRUCTION EQUIPMENT LOADS SUCH AS FORKLIFTS, MATERIAL STOCKPILES, ETC.

2. EVALUATION OF STRUCTURE FOR INSTALLATION OF ANY NECESSARY SHORING FOR MOVING LOADS DURING INSTALLATION OF HEAVY EQUIPMENT.
4. WHERE DIMENSIONS OR WEIGHTS OF EQUIPMENT OR SYSTEMS ARE VARIABLE FROM MANUFACTURER TO MANUFACTURER, VERIFY DIMENSIONS AND WEIGHTS SHOWN ON DRAWINGS WITH SELECTED MANUFACTURER PRIOR TO ORDERING MATERIALS, NOTIFY ENGINEER OF DISCREPANCIES.
5. DO NOT SUSPEND POINT LOADS FROM ROOF SHEATHING OR ROOF PURLINS UNLESS APPROVED BY THE ENGINEER. POINT LOADS INCLUDE, BUT ARE NOT LIMITED TO: HANGERS FOR CEILINGS, PIPES, DUCTS, STEEL STUDS, EQUIPMENT, ETC. CONTRACTOR INSTALLING SUCH POINT LOADS SHALL PROVIDE SUB-FRAMING TO TRANSFER LOAD TO THE STRUCTURE SUPPORTING DECK.

MATERIAL DESIGN PROPERTIES:

CONCRETE PROPERTIES:

1. ALL CONCRETE DESIGN AND CONSTRUCTION SHALL CONFORM TO THE LOCAL BUILDING CODE REQUIREMENTS AND THOSE OF THE LATEST EDITION OF THE FOLLOWING STANDARDS: ACI 318, ACI 315, ACI 301, AND ACI 305 # 306.
- | USE | 28 DAY STRENGTH | MIN. H2O /CEMENT RATIO | SLUMP (INCHES) | MAX. AGGREGATE SZ. |
|-----------------|-----------------|------------------------|----------------|--------------------|
| INTERIOR FLOORS | 3,500 PSI | .62 | 3 ±1 | 3/4 |
| WALLS | 3,500 PSI | .62 | 3 ±1 | 3/4 |
| PIERS | 3,500 PSI | .62 | 3 ±1 | 3/4 |
| FOOTINGS | 3,500 PSI | .62 | 3 ±1 | 1-1/2 |
| EXTERIOR FLOORS | 4,000 PSI | .48 | 4 ±1 | 3/4 |
2. ALL CONCRETE SUBJECT TO EXTERIOR EXPOSURE SHALL BE AIR ENTRAINED TO 6% (+/- 1.5%) AND HAVE A MAXIMUM 3/4" AGGREGATE. ALL CONCRETE WITHOUT UPERPLASTICIZERS SHALL HAVE A MAXIMUM SLUMP OF 4" ± 1".

REINFORCING STEEL STRENGTHS:

BARS (ASTM A615, GRADE 60) fy = 60,000 PSI
WELDED WIRE MESH (ASTM A 185) fy = 65,000 PSI

STRUCTURAL STEEL STRENGTHS:

OTHER:
W SHAPES (ASTM A992, GR50) fy = 50,000 PSI
ANGLES, CHANNELS, PLATES, # BARS (ASTM A36) fy = 36,000 PSI
SQUARE # RECTANGULAR TS OR HS5 SECTIONS (ASTM A500 ,GR B) fy = 42,000 PSI
HIGH STRENGTH BOLTS (ASTM A325)

| FROST WALL SCHEDULE | | | | | | |
|---------------------|------------|---------|-------|------------------------------------------------------------------|-------------|---------|
| MARK | DIMENSIONS | | | REINFORCEMENT | TOP OF ELEV | REMARKS |
| | WIDTH | LENGTH | DEPTH | | | |
| PIER 1 | 18" | 24" | 4'-0" | (10) #6 VERTICAL BARS W/ #3 TIES @ 12" O.C. TOP 4 TIES @ 3' O.C. | 100'-0" | |
| PIER 2 | 18" | 17 1/2" | 4'-0" | (8) #6 VERTICAL BARS W/ #3 TIES @ 12" O.C. TOP 4 TIES @ 3' O.C. | 100'-0" | |
| PIER 3 | 19" | 22" | 4'-0" | (12) #6 VERTICAL BARS W/ #3 TIES @ 12" O.C. TOP 4 TIES @ 3' O.C. | 100'-0" | |
| PIER 4 | 19" | 22" | 4'-0" | (12) #6 VERTICAL BARS W/ #3 TIES @ 12" O.C. TOP 4 TIES @ 3' O.C. | 100'-0" | |
| PIER 5 | 19" | 14" | 4'-0" | (8) #6 VERTICAL BARS W/ #3 TIES @ 12" O.C. TOP 4 TIES @ 3' O.C. | 100'-0" | |

NOTE: PRIOR TO CONSTRUCTION CONTRACTOR TO VERIFY PIER SIZES WITH METAL BUILDING MFG'S BASE PLATE AND ANCHOR BOLT LAYOUTS

| FOOTING SCHEDULE | | | | | | |
|------------------|------------|--------|-------|------------------------------------------------|-------------|---------|
| MARK | DIMENSIONS | | | REINFORCEMENT (W)- SPAN WIDTH (L)- SPAN LENGTH | TOP OF ELEV | REMARKS |
| | WIDTH | LENGTH | DEPTH | | | |
| F1 | 6'-0" | 6'-0" | 1'-0" | #5 BARS @ 12" O.C. , E.W. | 96'-0" | |
| F2 | 4'-0" | 4'-0" | 1'-0" | #5 BARS @ 12" O.C. , E.W. | 96'-0" | |
| F3 | 7'-0" | 5'-0" | 1'-0" | #5 BARS @ 12" O.C. , E.W. | 96'-0" | |
| STRIP 16" | 1'-4" | CONT. | 1'-0" | (2) #5 BARS, CONT. | 96'-0" | |

STRUCTURAL DESIGN DATA:

DESIGN CODE:

2025 WISCONSIN ENROLLED COMMERCIAL BUILDING CODE (2021 IBC)

SOIL LOAD:

ALLOWABLE NET SOIL BEARING PRESSURE (ASSUMED) 2,000 PSF
SOILS REPORT AVAILABLE NO

*SEISMIC LOAD:

SEISMIC USE GROUP / RISK CATEGORY II
SEISMIC LOAD IMPORTANCE FACTOR (Ie) I
SEISMIC SITE CLASS D (ASSUMED)
MAPPED SPECTRAL RESPONSE ACCELERATION (Ss) 0.072
MAPPED SPECTRAL RESPONSE ACCELERATION (Si) 0.047
SPECTRAL RESPONSE COEFFICIENT (Sds) 0.077
SPECTRAL RESPONSE COEFFICIENT (Sd1) 0.075
SEISMIC DESIGN CATEGORY A
SEISMIC RESPONSE COEFF. CS 0.026
RESPONSE MODIFICATION COEFF. 3.00
LONGITUDINAL BASE SHEAR 0.026 x W KIP5
TRANSVERSE BASE SHEAR 0.026 x W KIP5

*WIND LOAD:

ULTIMATE WIND SPEED 115 MPH (Vult)
NOMINAL WIND SPEED 89 MPH (Vasd)
RISK CATEGORY II-STANDARD BUILDINGS
WIND EXPOSURE C
INTERNAL PRESSURE COEFFICIENTS ± 0.18

ROOF DESIGN LOAD:

SNOW ROOF LOAD 21.0 PSF
GROUND SNOW LOAD 30.0 PSF
ROOF LIVE LOAD 20.0 PSF
ROOF DEAD LOAD PER MFG
COLLATERAL LOAD 5.0 PSF
UNBALANCED LOAD: N/A
DRIFT LOADS SEE APPROPRIATE DIAGRAMS ON 52.0

*SNOW LOAD:

GROUND SNOW LOAD 30 PSF
SNOW EXPOSURE FACTOR (Ce) 1.0
SNOW IMPORTANCE FACTOR (Is) 1.0
THERMAL FACTOR (Ct) 1.0
RISK CATEGORY II

* SEISMIC, WIND, AND SNOW LOAD CALCULATIONS AND DESIGN DATA SHALL BE PERFORMED AND SUPPLIED BY THE TRUSS MANUFACTURER.

FOUNDATION PLAN NOTES:

1. CONTRACTOR SHALL PROVIDE FROST PROTECTION AND MOISTURE PROTECTION FOR FOOTINGS EXPOSED DURING CONSTRUCTION.
2. REFER TO ARCHITECTURAL DRAWINGS OR PLUMBING DRAWINGS FOR SPECIFIC FLOOR DRAIN LOCATIONS AND ELEVATIONS.
3. REFER TO STRUCTURAL DETAIL PLAN SHEETS FOR MISCELLANEOUS DETAILS NOT INDICATED ON PLAN.
4. NOTIFY ENGINEER OF ANY UNUSUAL SOIL CONDITIONS. ALL FOOTINGS SHALL REST ON UNDISTURBED ROCK OR SOIL EXCAVATIONS FOR FOOTINGS SHALL BE APPROVED BY ENGINEER PRIOR TO PLACEMENT OF CONCRETE.
5. WHERE REQUIRED, REMOVE UNSUITABLE EXISTING SOILS BELOW FOOTINGS, SLABS-ON-GRADE, ETC. TO APPROVED BEARING SOIL. REPLACE WITH ENGINEERED FILL (COMPACTED TO 95% OF THE MODIFIED PROCTOR DENSITY) TO THE REQUIRED FOOTING BEARING ELEVATION. REVIEW SOIL REPORT, IF ANY. FILL MATERIAL SHALL HAVE A MINIMUM BEARING CAPACITY AS INDICATED IN THE STRUCTURAL DESIGN DATA SOIL LOAD INFORMATION ON SHEET 50.0. TYPE OF FILL MATERIAL AND PLACEMENT SHALL CONFORM TO SPECIFICATIONS UNDER THE DIRECTION AND SUPERVISION OF THE SOILS ENGINEER. SOILS ENGINEER SHALL FIELD VERIFY ALL BEARING CAPACITIES BEFORE FOOTINGS ARE POURED. CONTACT ENGINEER IF UNABLE TO ATTAIN LISTED SOIL BEARING PRESSURE.
6. PROVIDE A MINIMUM OF 8 INCHES OF WELL COMPACTED GRANULAR FILL BELOW ALL SLABS ON GRADE. COMPACT TO 95% OF THE MODIFIED PROCTOR DENSITY.
7. CONCRETE EXPOSED TO WEATHER (RETAINING WALLS, EXTERIOR SLABS, WALKS, CURBS, ETC. BUT EXCLUDING EXPOSED FOUNDATION WALLS) SHALL CONTAIN 4 TO 7 PERCENT AIR BY VOLUME.
8. DELIVERY TICKETS FOR EACH LOAD OF CONCRETE DELIVERED TO THE JOB SITE SHALL BE FURNISHED UPON REQUEST TO THE ENGINEER. TICKET INFORMATION SHALL CONTAIN ALL PERTINENT DESIGN INFORMATION, INCLUDING AMOUNT OF WATER ADDED AT THE JOB SITE, IF ANY.
9. VERIFY PIER CENTERLINE SPACINGS, ANCHOR BOLT LAYOUT, AND FOUNDATION DIMENSIONS WITH METAL BUILDING MANUFACTURER'S ANCHOR BOLT PLAN. VERIFY THAT ALL BASE PLATES WILL BEAR FULLY ON CONCRETE PIERS, NOTIFY ENGINEER OF ANY DISCREPANCIES PRIOR TO POURING CONCRETE.
10. FORMWORK FOR FOOTINGS SHALL CONSIST OF A MANUFACTURED FORM SYSTEM OR A MINIMUM 1-1/2" THICK WOOD PLANK SECURED TO WOOD OR STEEL STAKES. POURING TO EXCAVATION BANK MAY NOT BE DONE WITHOUT PRIOR APPROVAL OF THE ENGINEER.
11. MIXING AND PLACING OF CONCRETE TO BE IN ACCORDANCE WITH ACI 318. CONCRETE SHALL BE DEPOSITED AS NEARLY AS PRACTICAL IN ITS FINAL POSITION TO AVOID SEGREGATION DUE TO REHANDLING OR FLOWING. CONCRETING SHALL BE CARRIED ON A SUCH A RATE THAT CONCRETE IS AT ALL TIMES PLASTIC AND FLOWS READILY INTO SPACES BETWEEN REINFORCEMENT. ALL CONCRETE SHALL BE THOROUGHLY CONSOLIDATED BY SUITABLE MEANS DURING PLACEMENT AND SHALL BE THOROUGHLY WORKED AROUND REINFORCEMENT AND EMBEDDED FIXTURES AND INTO CORNERS OF FORMS.

NO. DATE

REVISIONS

SHEET NAME:

ADDRESS:

PROJECT:

2025 BUILDING ADDITION

UPJ MANUFACTURING

PROJECT NO: 0924-25
PLOT DATE: 11/1/25
PLOT BY: SSR
PLOT SCALE: As indicated

SHEET #

BID DOCUMENTS

STRUCTURAL NOTES

S87W35C670

GODFREY LN

EAGLE, WI 53119

2025 BUILDING ADDITION

UPJ MANUFACTURING

PROJECT NO: 0924-25
PLOT DATE: 11/1/25
PLOT BY: SSR
PLOT SCALE: As indicated

SHEET #

BID DOCUMENTS





D250SCA Bar Lock® Coupler

DESCRIPTION

The D250SCA Bar Lock is a rebar coupler consisting of a thick walled tube, specially designed lock shear bolts, serrated grip rails, and a center stop pin. The D250SCA is made from USA melted and rolled steel.

APPLICATION

The D250SCA Bar Lock Coupler is used to provide a continuous path of mechanical reinforcing between two pieces of the same diameter rebar.

PRODUCT SPECIFICATION

- Rebar sizes #3 (13mm) through #18 (57mm)
- Available in plain, epoxy coated or galvanized finish (call for availability for galvanized finish)
- Designed to achieve Type I splice when used with grade 60 rebar per ACI 318-17 and earlier
- Achieves Type 2 grade 60 splice per ACI 318-19 code when used with plain #4-#14 grade 60 rebar.



FEATURES

- Quick and easy installation
- No bar end preparation
- Installation at the job site
- Used in tension, compression and seismic applications

BENEFITS

- Saves time and money
- No fabrication required
- One product for all applications

| TECHNICAL DATA | | | | | | | | | |
|----------------|------------|--------------------------|------------|-------------|--------------|-------------------|--------------|---------------|---------------|
| Base resin | | Fiberglass reinforcement | | | | Soft and hardener | | Shrinkage | |
| Part | Weight (g) | Length (cm) | Width (cm) | Height (cm) | Volume (cm³) | Weight (g) | Volume (cm³) | Shrinkage (%) | Shrinkage (%) |
| 101 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 102 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 103 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 104 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 105 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 106 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 107 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 108 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 109 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 110 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 111 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 112 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 113 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 114 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 115 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 116 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 117 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 118 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 119 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |
| 120 | 100 | 1.3 | 3.9 | 1.24 | 1.56 | 1 | 0.1 | 40 | 30 |

APPROVALS / COMPLIANCE

- ACI 318 Type 1 or ACI 318-19 Type 2 (ASTM A615 or A706, Grade 60)
- IAPMO UES Listed (Er-319)
- AASHTO
- International Build Code (IBC)

INSTALLATION

1. Insert end of the first bar halfway into the coupler to the center pin. Hold bar in place and hand-tighten all bolts.
2. Insert end of the second bar halfway into the coupler to the center pin. Hold bar in place and hand-tighten all bolts.
3. In a random alternating pattern, tighten all bolts to approximately 50% of the specified bolt torque value.
4. In a random alternating pattern, tighten all bolts to approximately 75% of the specified bolt torque value.
5. Tighten all bolts in a random alternating pattern until all bolt heads shear off.

Notes

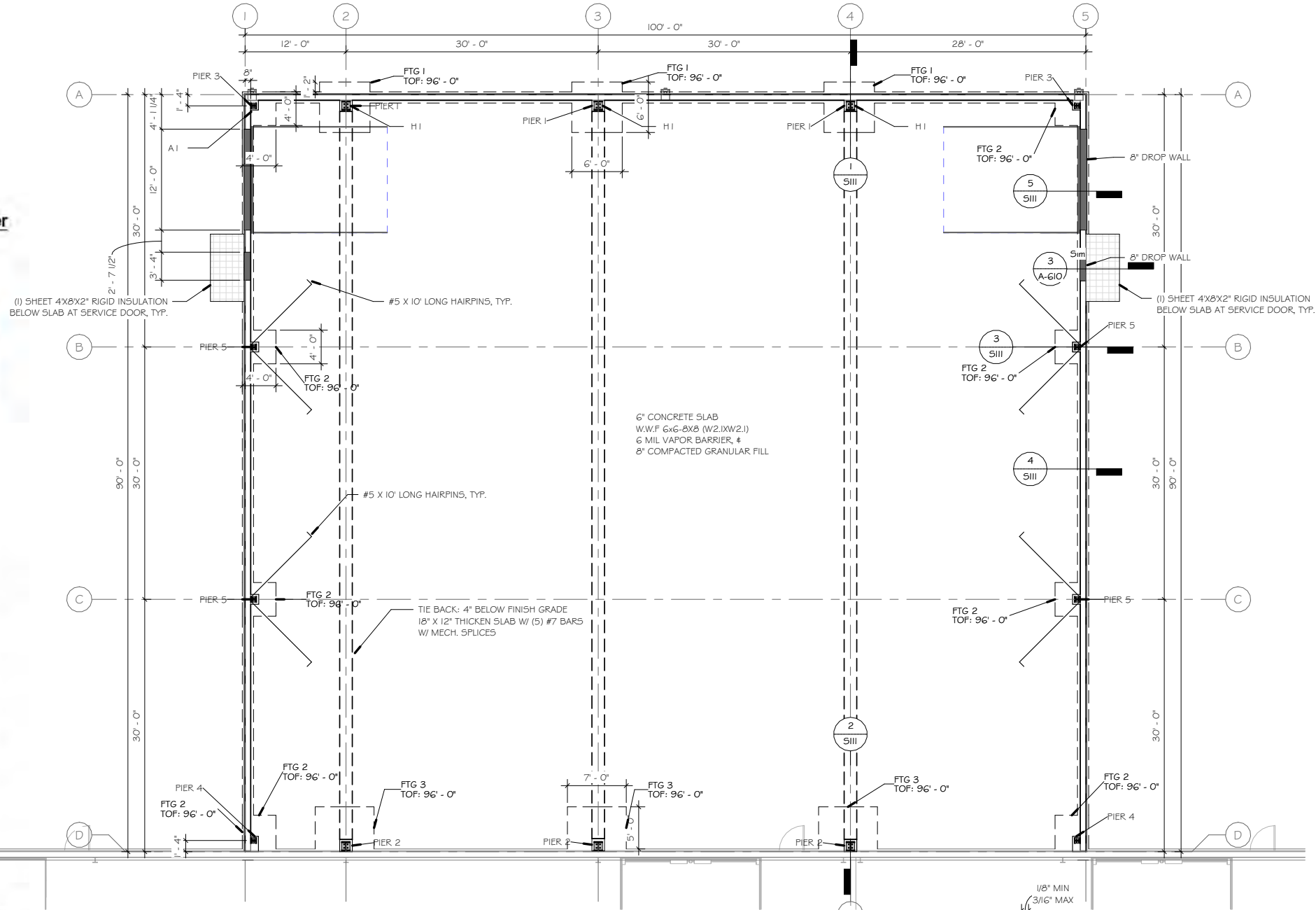
A. Prior to tightening, the serrated rails MU remain aligned in the same position as they were manufactured. If they are damaged or knocked out of alignment while positioning, installation **MUST** cease, and a new coupler used.

B. Bolt tightening **MUST** be done in a rand alternating pattern similar to tightening the lug nuts on an automobile wheel. The torque percentages are just approximations, the main intent is to torque the bolt heads off in 3 passes.

C. A high-quality, 1" -pneumatic drive, impact wrench with at least 100 psig air flow and 185 CFM of delivered air through a no less than 0.75" hose MUST be used for installation for sizes #8 thru #18. Sizes #4 thru #7 may be installed with smaller impact wrenches.

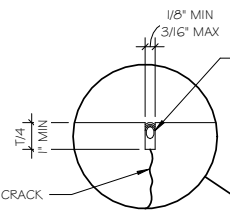
Page 1 of 2

File Date: 8/12/2021

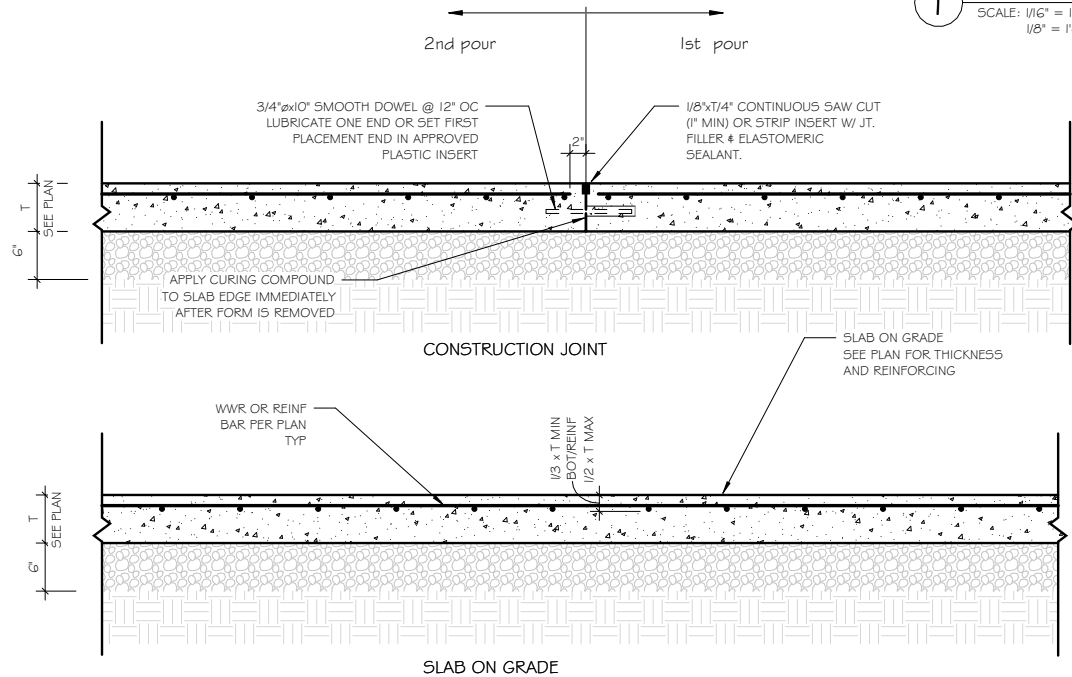


 FOUNDATION PLAN

SCALE: 1/16" = 1'-0" on 11" x 17"
1/8" = 1'-0" on 22" x 34"

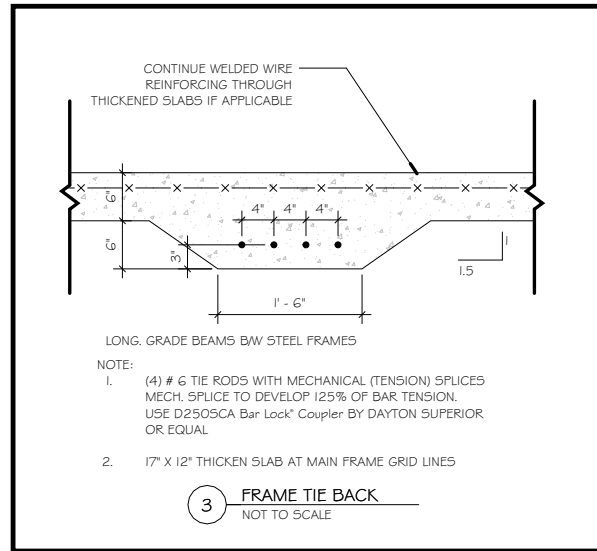


1/8"xT/4" CONTINUOUS
(1" MIN) OR STRIP INS
FILLER & ELASTOMER
SEALANT.



NOTES:

1. SAW CUT JOINTS WITH EARLY ENTRY SAW AS SOON AS THE SLAB CAN SUPPORT THE WEIGHT OF THE SAW CUTTING EQUIPMENT AND CUTTING DOES NOT DAMAGE THE SLAB.
2. INSTALL JOINTS INDICATED ON PLANS UNO. SUBMIT PROPOSED ALTERNATE LOCATIONS OR ADDITIONAL JOINTS NOT SHOWN TO THE DOR FOR APPROVAL. PROPOSED ALTERNATE LOCATIONS MAY BE REJECTED. MAX JOINT SPACING MUST BE BETWEEN (24 TO 36) x SLAB THICKNESS. MAX JOINT LENGTH MUST BE 10'-0" BETWEEN CONSTRUCTION JOINTS. RATIO OF LENGTH TO WIDTH MUST NOT EXCEED 4:1. TOTAL AREA OF POUR MUST NOT EXCEED 10,000 SF BETWEEN CONSTRUCTION JOINTS.
3. DISCONTINUE REINFORCEMENT AT CONSTRUCTION JOINTS.



FRAME TIE BACK
NOT TO SCALE

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| REVISIONS | NO | DATE |
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|-----------|----|------|

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PROJECT:



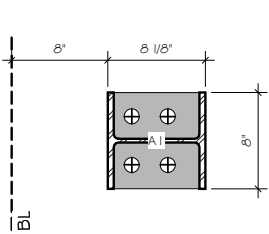
**ADVANCED
BUILDING**
CORPORATION

PROJECT NO: 0924-25
PLOT DATE : 11/1/25
PLOT BY : SSR
PLOT SCALE : As indicated

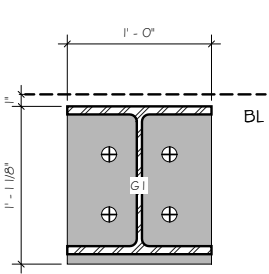
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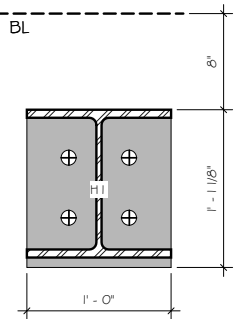
BASE PLATES



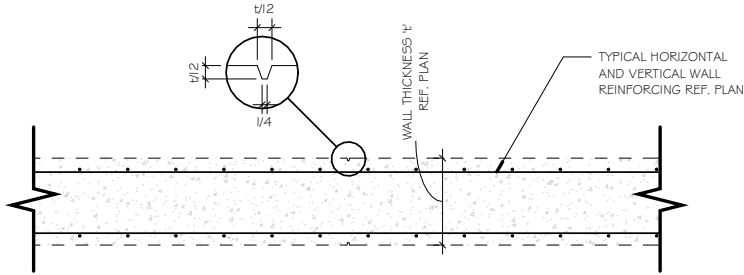
AI, BI, CI, DI, EI, FI
GRID LINE A, B, C, D
BASE PLATE THICKNESS PER MFG.
(4)~3/4" DIA. J-BOLTS GR36 #
W/ MIN. 12" EMBEDMENT
ELEV.=100'-0"



GI (MAIN FRAME)
BASE PLATE THICKNESS PER MFG.
GI (4)~1" DIA. Ø THREADED ROD
F1554 GR36 W/ DOUBLE NUT ON END
W/ MIN. 12" EMBEDMENT.
ELEV.=100'-0"

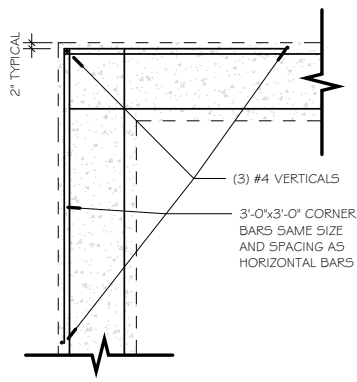


HI (MAIN FRAME)
BASE PLATE THICKNESS PER MFG.
BP: HI (4)~1" DIA. Ø THREADED ROD F1554
GR36 W/ DOUBLE NUT ON END # W/ MIN. 12"
EMBEDMENT.
ELEV.=100'-0"

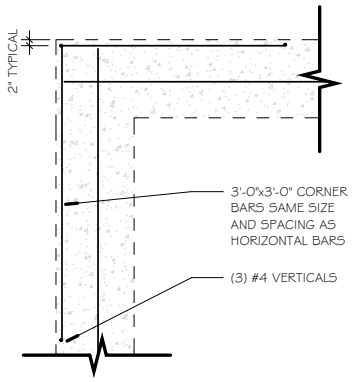


NOTE: PROVIDE FND WALL CONTOL JOINTS @ 30' SPACING.

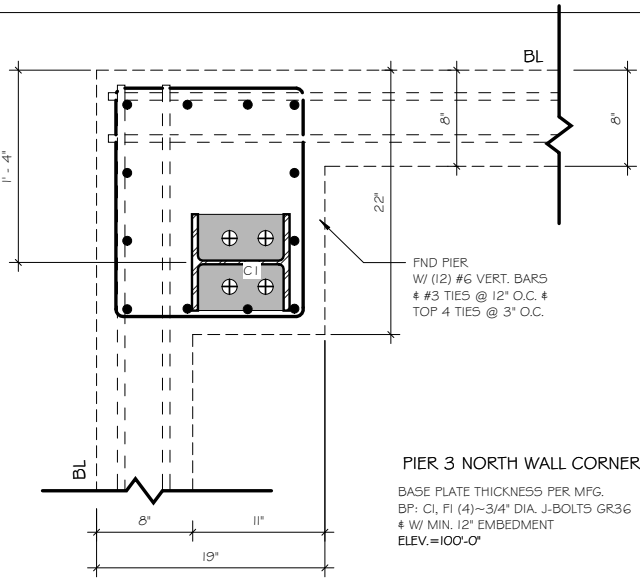
6 FND WALL CJ
NOT TO SCALE



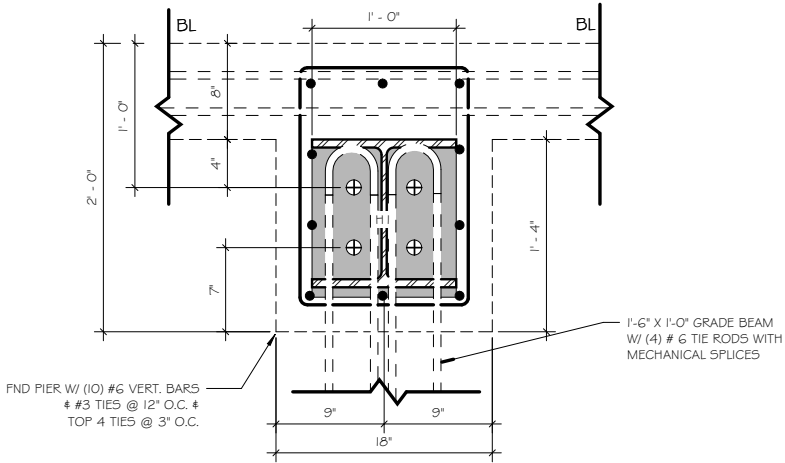
4 FND WALL CORNERS
NOT TO SCALE



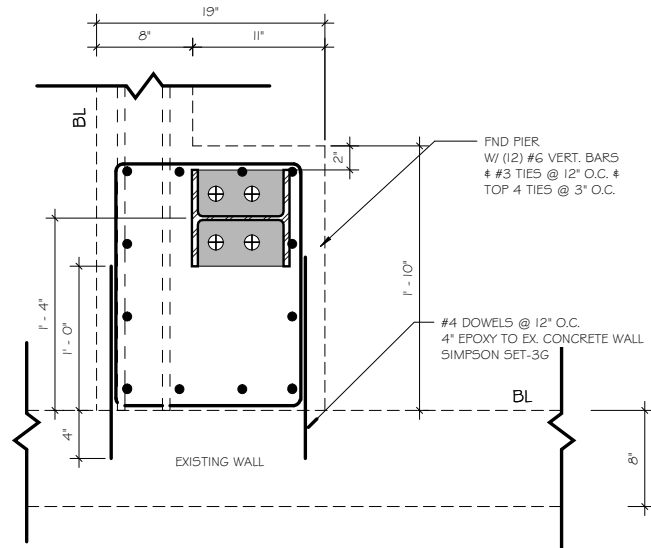
PIER DETAILS



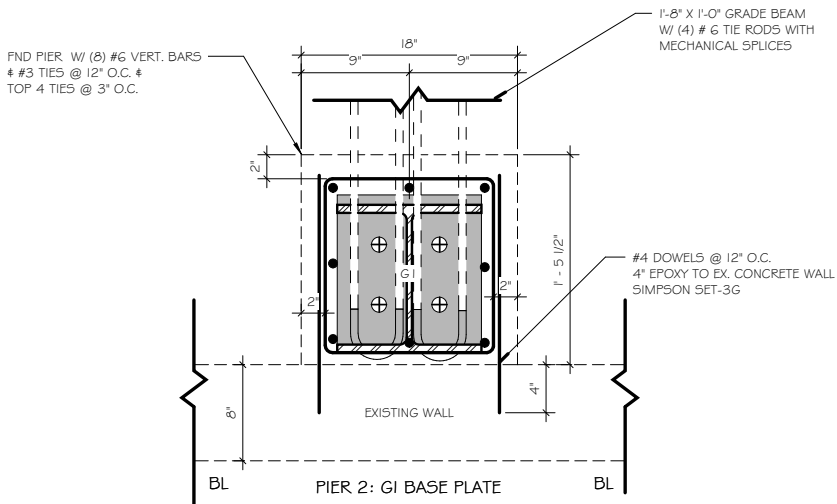
PIER 3 NORTH WALL CORNERS
BASE PLATE THICKNESS PER MFG.
BP: CI, FI (4)~3/4" DIA. J-BOLTS GR36
W/ MIN. 12" EMBEDMENT
ELEV.=100'-0"



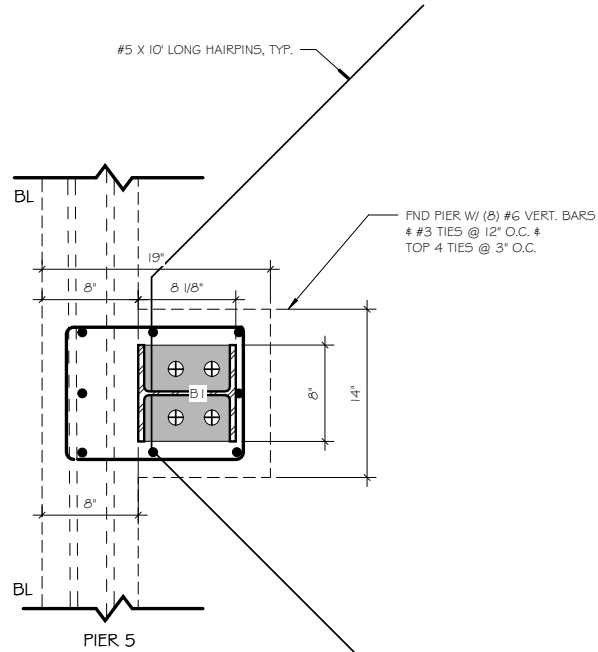
PIER 1: BASE PLATE HI
BASE PLATE THICKNESS PER MFG.
BP: HI: (4)~1" DIA. Ø THREADED ROD F1554
GR36 W/ DOUBLE NUT ON END # W/ MIN. 12"
EMBEDMENT.
ELEV.=100'-0"



PIER 4 SOUTH WALL CORNERS
BASE PLATE THICKNESS PER MFG.
BP: AI# DI (4)~3/4" DIA. J-BOLTS GR36
W/ MIN. 12" EMBEDMENT
ELEV.=100'-0"



PIER 2: GI BASE PLATE
BASE PLATE THICKNESS PER MFG.
BP: GI: (4)~1" DIA. Ø THREADED ROD F1554
GR36 W/ DOUBLE NUT ON END # W/ MIN. 12"
EMBEDMENT.
ELEV.=100'-0"



PIER 5
BASE PLATE THICKNESS PER MFG.
BP: BI \$ EI (4)~3/4" DIA. J-BOLTS GR36
W/ MIN. 12" EMBEDMENT
ELEV.=100'-0"

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NO. DATE

BASE PLATE DETAILS

587W35670
GODFREY LN
EAGLE, WI 53119

2025 BUILDING ADDITION
UPI MANUFACTURING

ROUSSEV
ENGINEERING
SOLUTIONS, LLC
7563 Driftless Rdg Wy
Verona, WI 53593

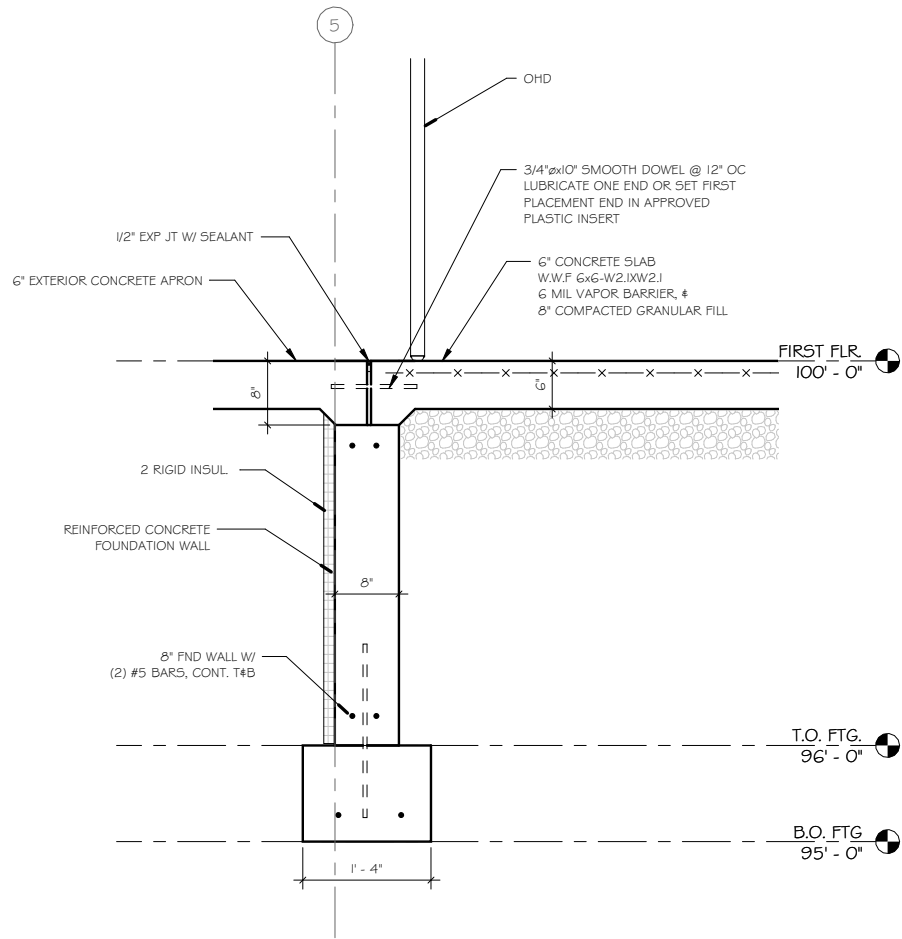
ADVANCED
BUILDING
CORPORATION
3802 KIPP ST.
MADISON, WI 53718

PROJECT NO: 0924-25
PLOT DATE: 11/1/25
PLOT BY: SSR
PLOT SCALE: As indicated

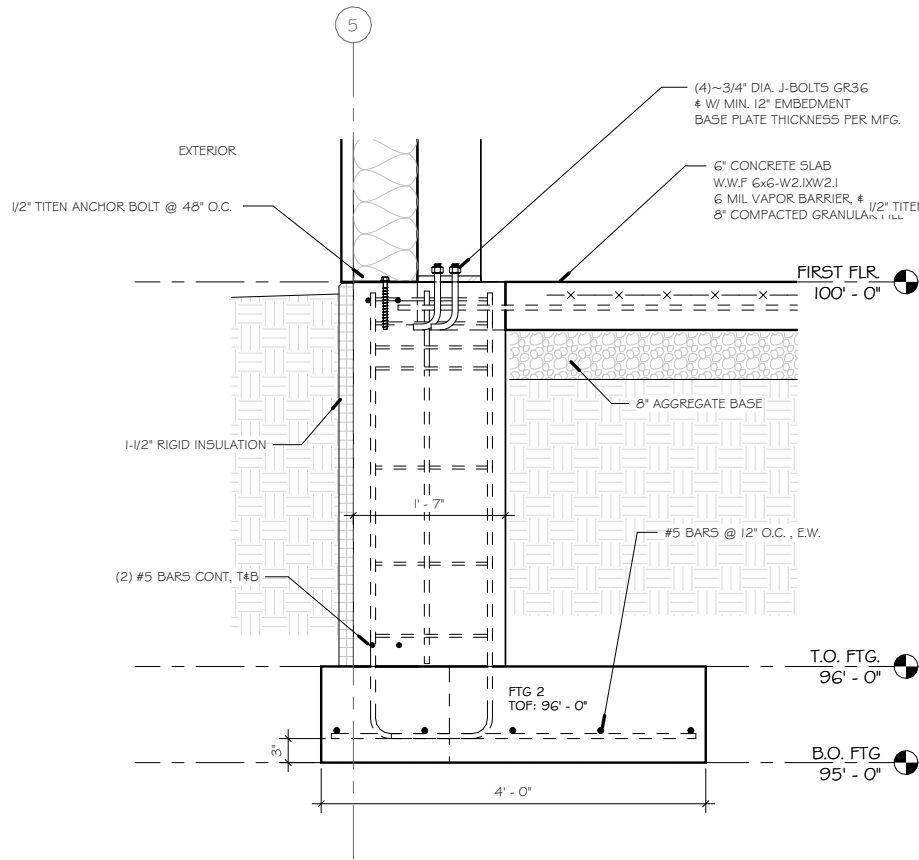
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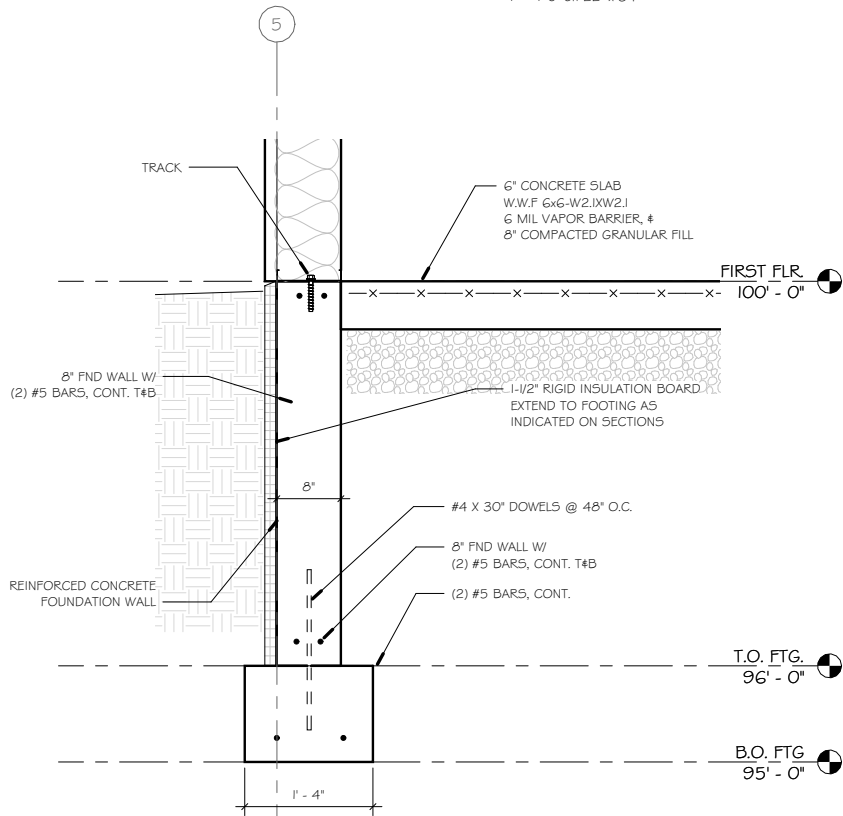
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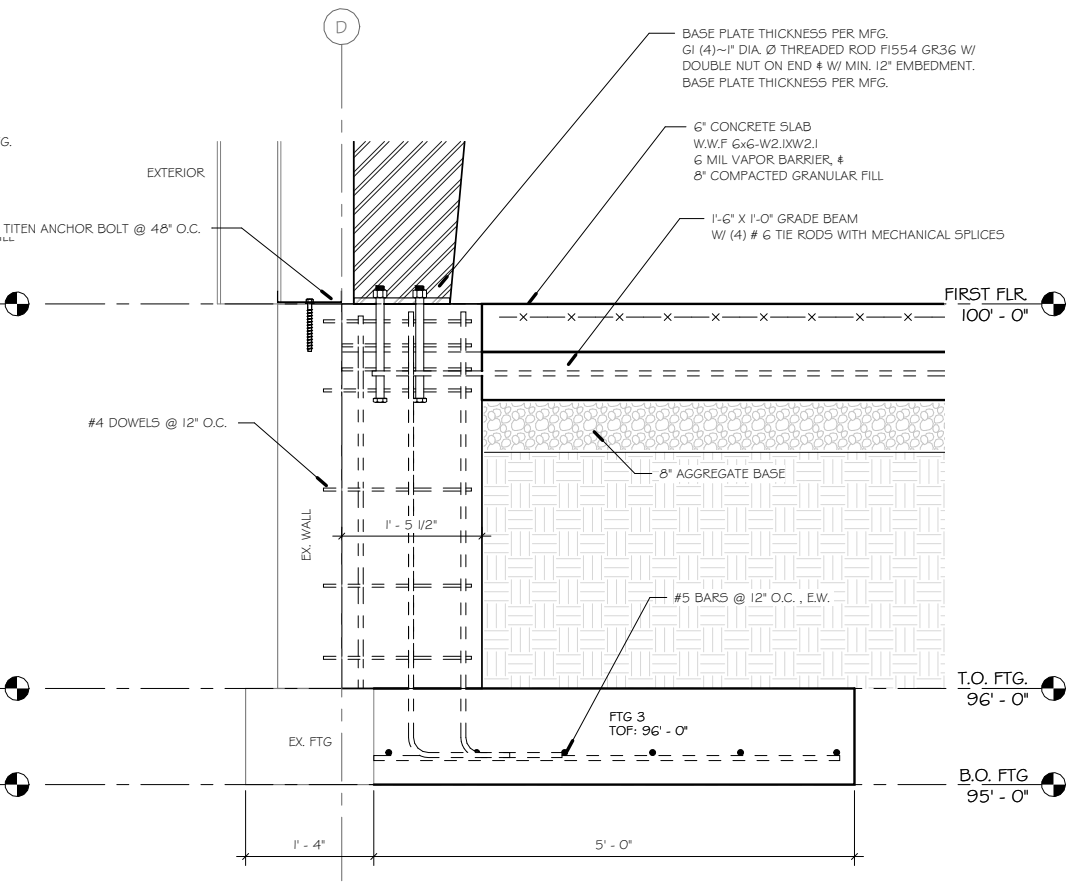
5 FND WALL @ OHD
SCALE: 1/2" = 1'-0" on 11" x 17"
1" = 1'-0" on 22" x 34"



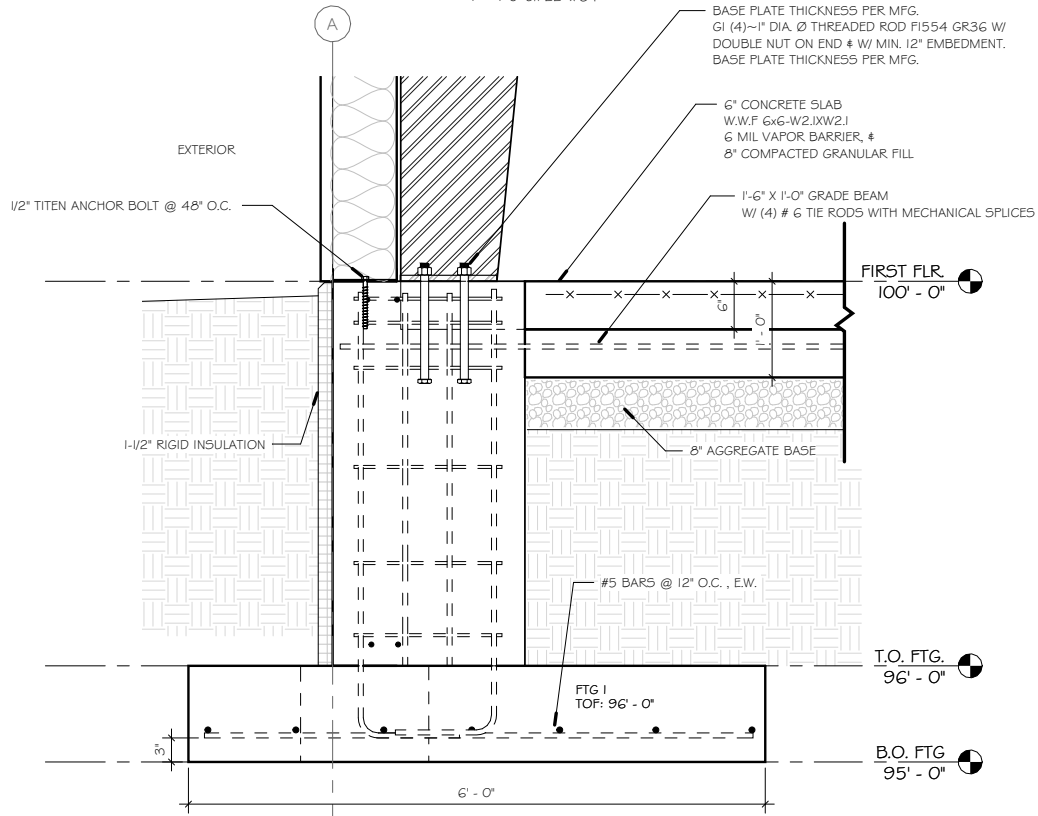
3 PIER 5 & FTG 2
SCALE: 1/2" = 1'-0" on 11" x 17"
1" = 1'-0" on 22" x 34"



4 TYP. FND WALL
SCALE: 1/2" = 1'-0" on 11" x 17"
1" = 1'-0" on 22" x 34"

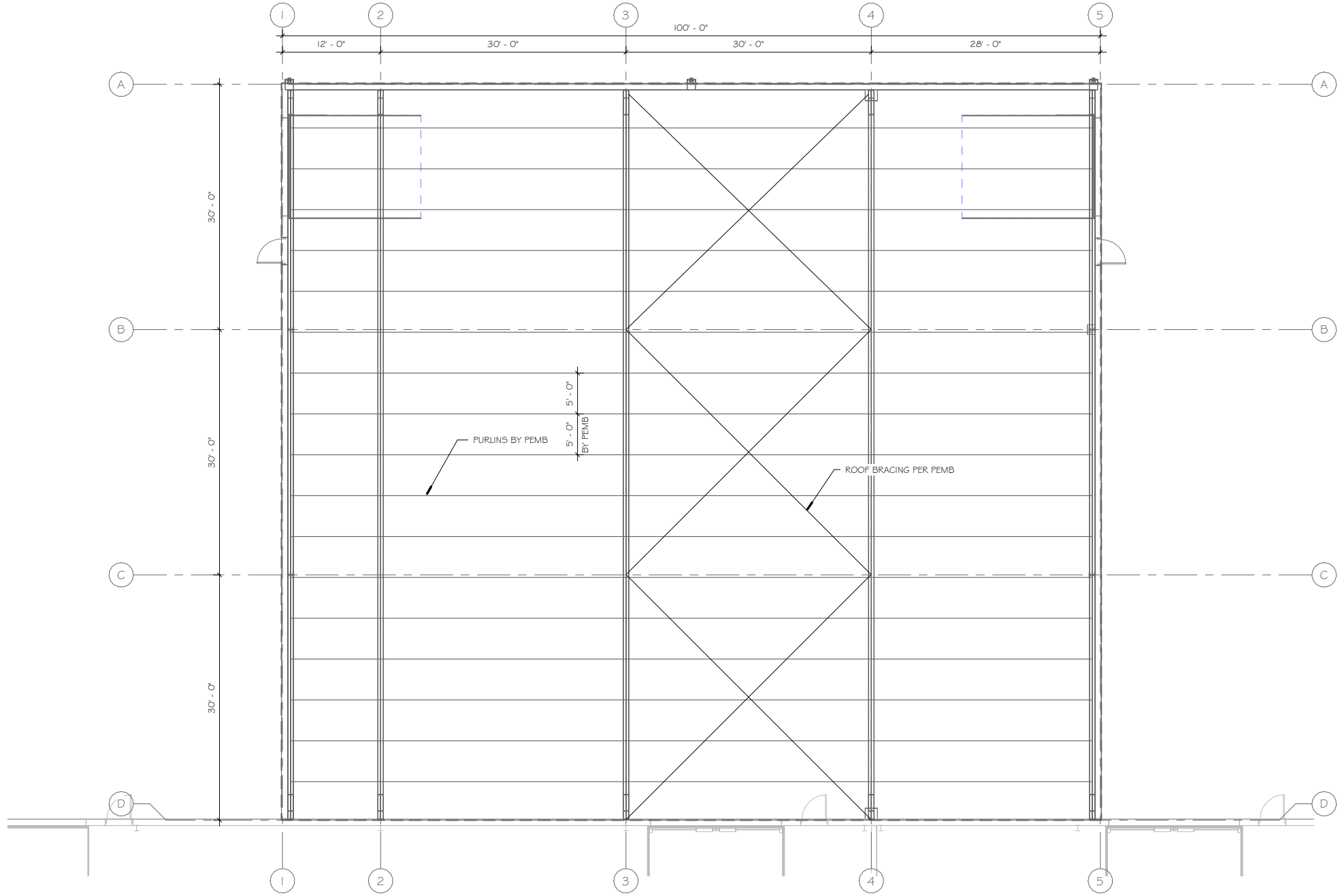


2 PIER 2 & FTG 1
SCALE: 1/2" = 1'-0" on 11" x 17"
1" = 1'-0" on 22" x 34"

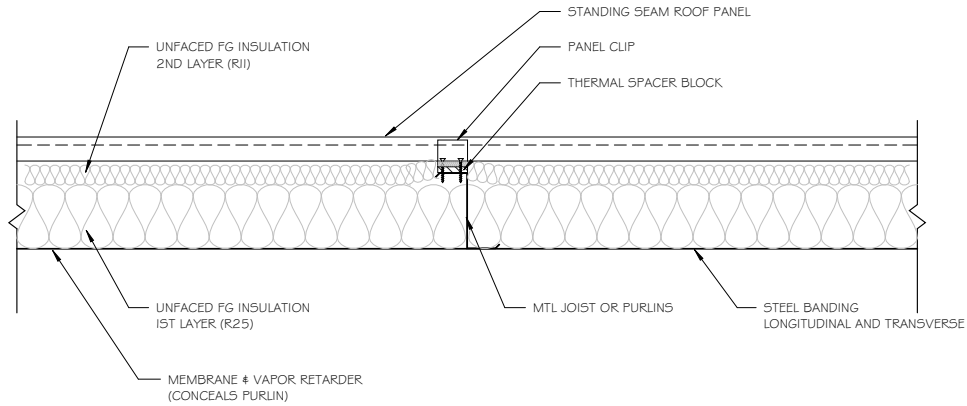


1 PIER 1 & FTG 1
SCALE: 1/2" = 1'-0" on 11" x 17"
1" = 1'-0" on 22" x 34"

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1 ROOF FRAMING PLAN
SCALE: 1/16" = 1'-0" on 11" x 17"
1/8" = 1'-0" on 22" x 34"



R25+R11 LINER SYSTEM ON METAL ROOF
U=0.031 PER ASHRAE 90.1, TABLE A2.3.3

2 ROOF SECTION
NOT TO SCALE



3802 KIPP ST.
MADISON, WI 53718

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SHEET # 5200

BID DOCUMENTS

PROJECT:
2025 BUILDING ADDITION
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608-620-3036 (Office) • svet@rousseveengineering.com

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EAGLE, WI 53119

SHEET NAME:
ROOF FRAMING PLAN

REVISIONS
NO. DATE