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Revision Schedule		
No.	Revision	Date

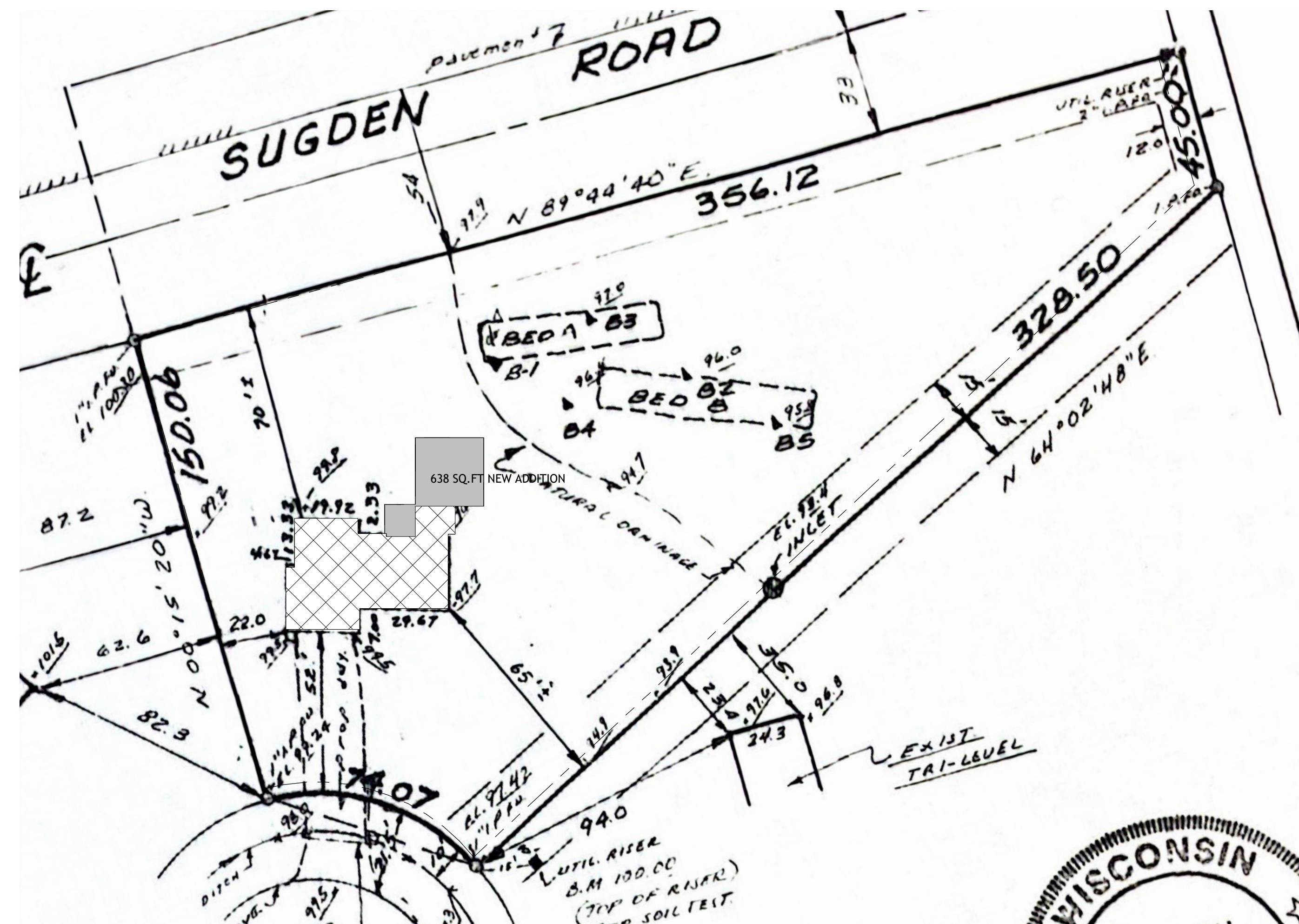
**RESIDENTIAL
ADDITION**
W326S7928 Memory Ln Ct
Mukwonago, WI 53149

Lighting Fixture Schedule - See Plan A101 and A102							
Mark	TYPE	Lamp	Count	Electrical Data	Fixture Material Finish	Manufacturer	Luminous Flux
	Recessed Lamp - Round - LED: 4" Trimmed Downlight	LED	23	120 V/1-20 VA			700 lm

Door Schedule				
Mark	Description	Width	Height	Count
14	Door-Interior-Single-2_Panel-Wood: 34" x 84"	2' - 10"	7' - 0"	1
15	Door-Interior-Single-2_Panel-Wood: 34" x 84"	2' - 10"	7' - 0"	1
16	Door-Interior-Single-2_Panel-Wood: 34" x 84"	2' - 10"	7' - 0"	1
17	Door-Interior-Single-2_Panel-Wood: 34" x 84"	2' - 10"	7' - 0"	1

Wall Schedule					
Type Mark	Width	Height	Length	Area	Type

WINDOWS SCHEDULE						
Mark	Window Description	Type	Width	Height	Count	Comments
8	Window-Double-Hung	30" x 46"	2' - 6"	3' - 10"	1	
12	Window-Double-Hung	30" x 46"	2' - 6"	3' - 10"	1	
28	Window-Double-Hung	30" x 46"	2' - 6"	3' - 10"	1	
9	Window-Single-Hung	34" x 56"	2' - 10"	4' - 8"	1	
10	Window-Single-Hung	34" x 56"	2' - 10"	4' - 8"	1	
11	Window-Single-Hung	34" x 56"	2' - 10"	4' - 8"	1	
25	Window-Single-Hung	34" x 56"	2' - 10"	4' - 8"	1	
26	Window-Single-Hung	34" x 56"	2' - 10"	4' - 8"	1	
27	Window-Single-Hung	34" x 56"	2' - 10"	4' - 8"	1	
Grand total						



1 SITE PLAN - PLAT
1" = 30'-0"

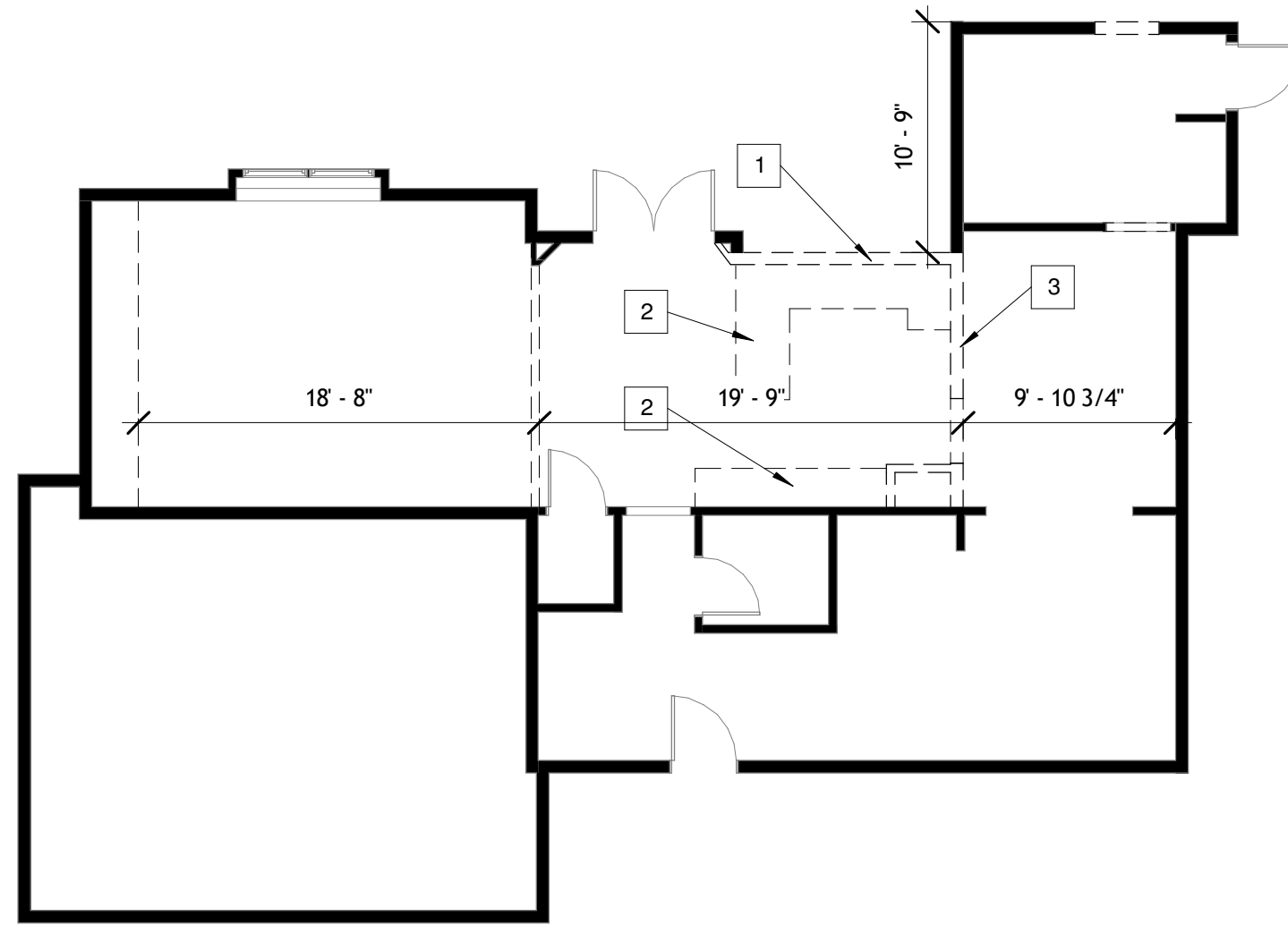
SCALE
VARIES



TITLE

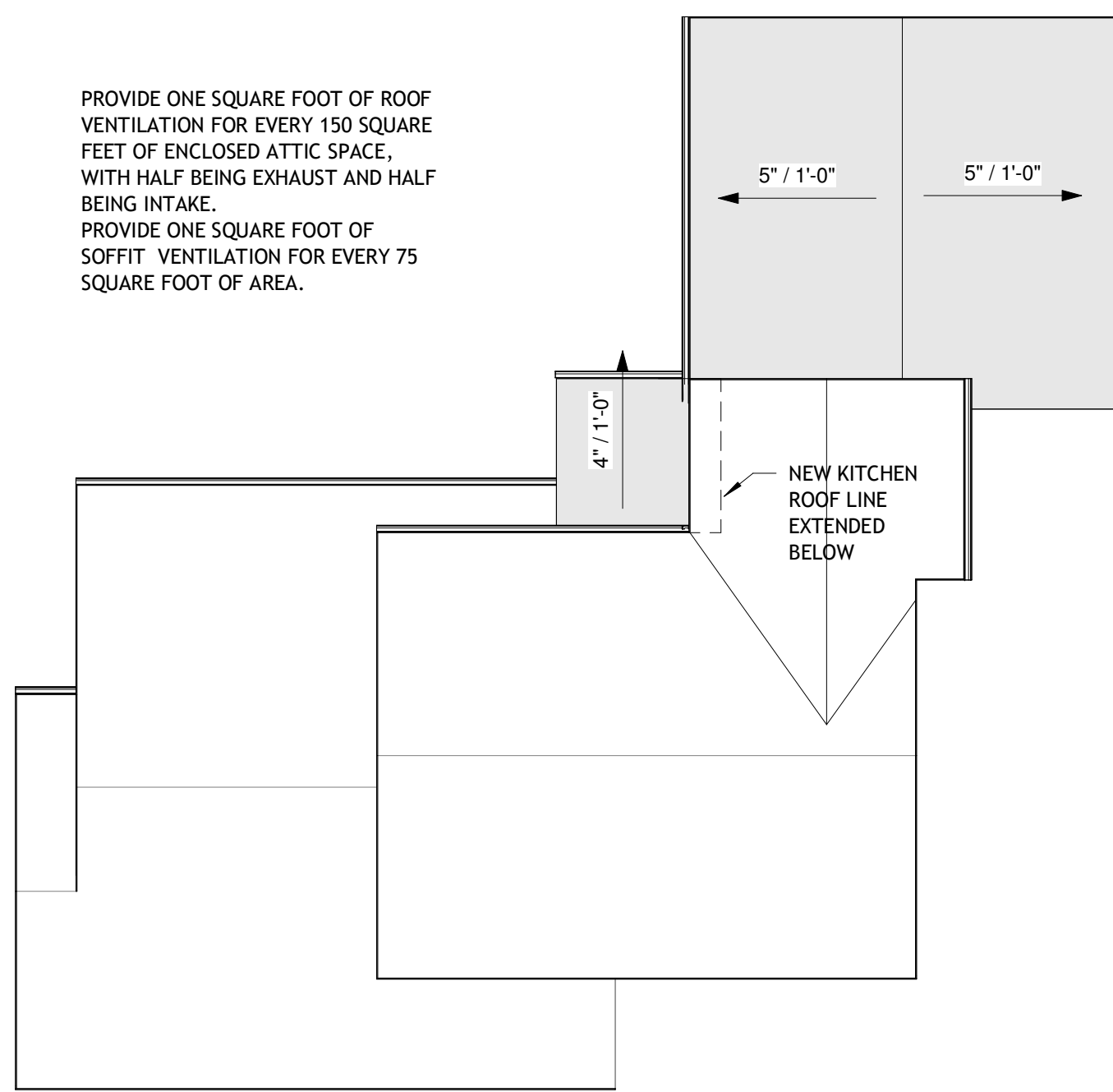
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Revision Schedule		
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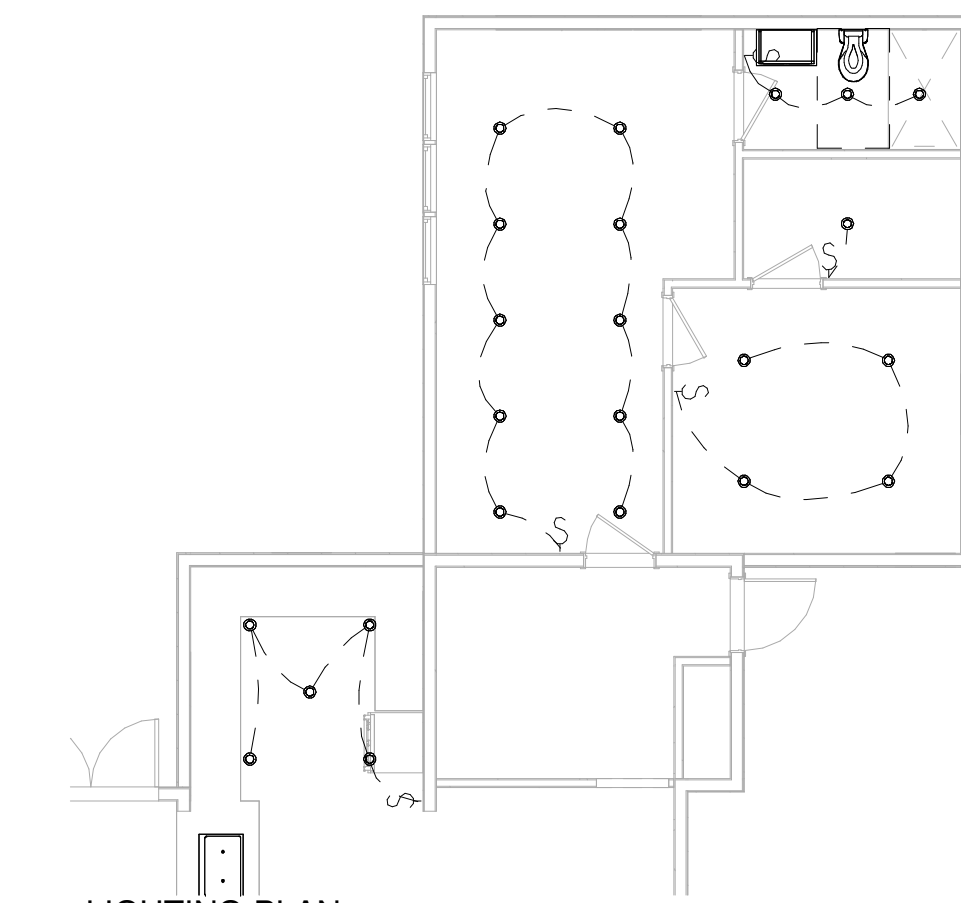


③ Level 1 Demo
1/8" = 1'-0"

PROVIDE ONE SQUARE FOOT OF ROOF VENTILATION FOR EVERY 150 SQUARE FEET OF ENCLOSED ATTIC SPACE, WITH HALF BEING EXHAUST AND HALF BEING INTAKE.
PROVIDE ONE SQUARE FOOT OF SOFFIT VENTILATION FOR EVERY 75 SQUARE FOOT OF AREA.



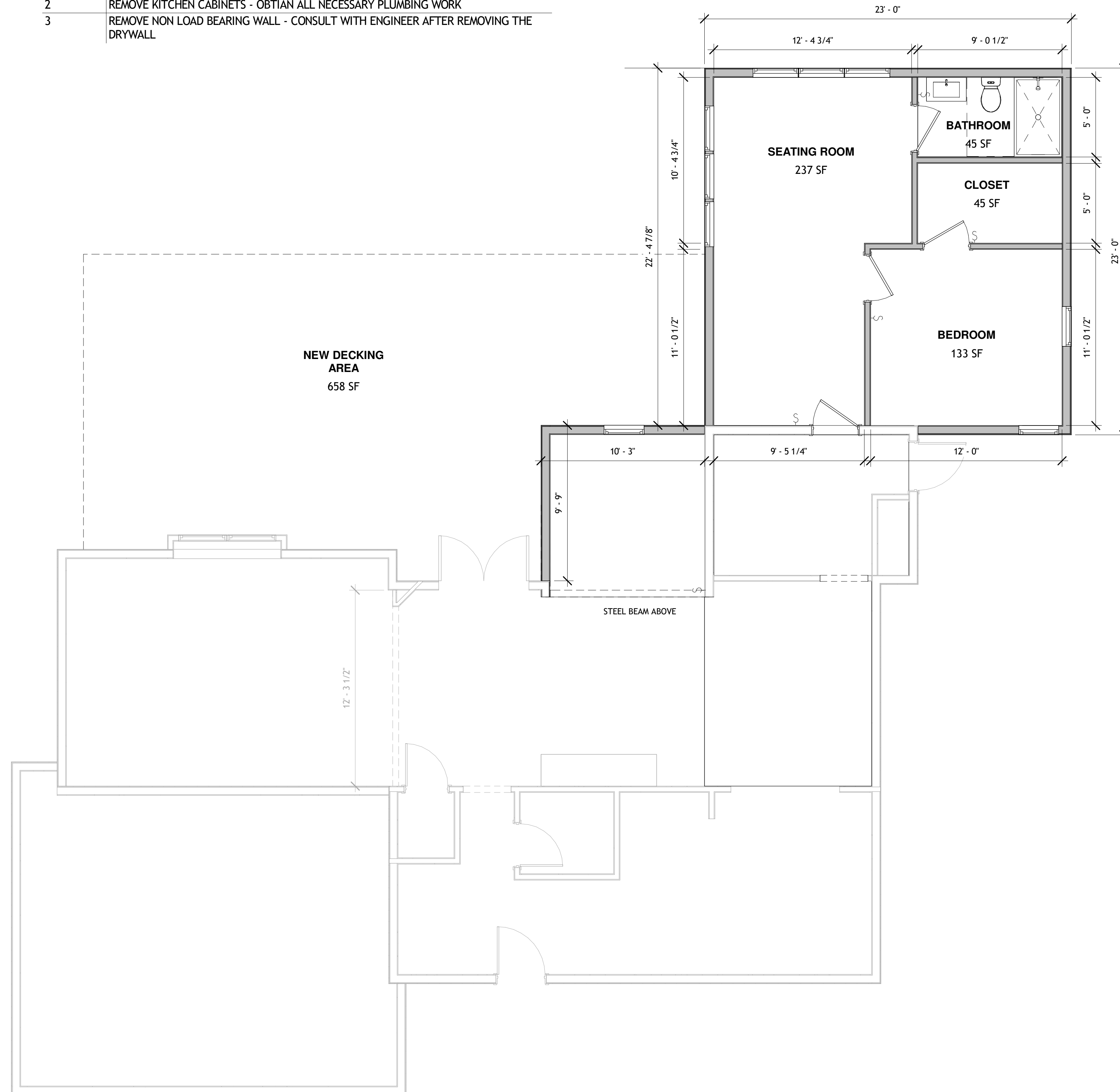
② Roof Plan
1" = 10'-0"



④ LIGHTING PLAN
1/8" = 1'-0"

DEMO NOTES

label number	label text
1	REMOVE LOAD BEARING WALL
2	REMOVE KITCHEN CABINETS - OBTAIN ALL NECESSARY PLUMBING WORK
3	REMOVE NON LOAD BEARING WALL - CONSULT WITH ENGINEER AFTER REMOVING THE DRYWALL



① Level 1
1/4" = 1'-0"

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SCALE
VARIES



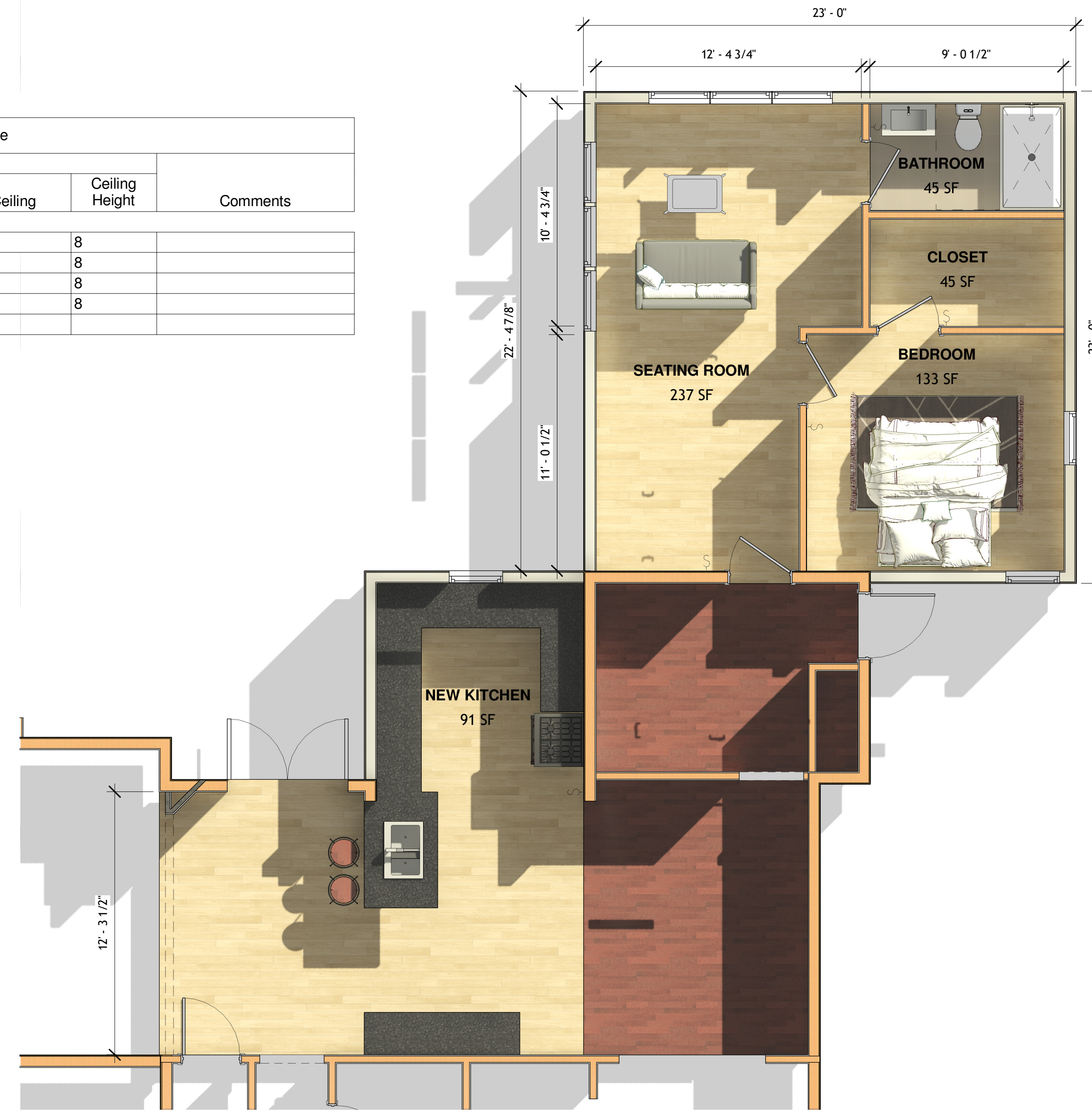
ARCHITECTURAL

A1

Revision Schedule

No.	Revision	Date

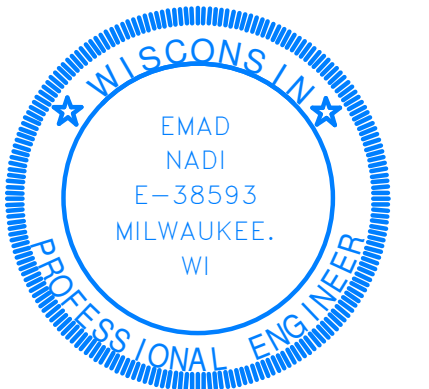
Room Finish Schedule						
Room Name	Area	Finish			Ceiling Height	Comments
		Floor	Wall	Ceiling		
SEATING ROOM	237 SF	LAMINATE	GWB	GWB	8	
BEDROOM	133 SF	LAMINATE	GWB	GWB	8	
BATHROOM	45 SF	TILES	GWB	GWB	8	
CLOSET	45 SF	LAMINATE	GWB	GWB	8	
NEW KITCHEN	91 SF	LAMINATE	GWB	GWB		



① Level 1 - Furniture
1/4" = 1'-0"

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W326S7928 Memory Ln Ct
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SCALE
VARIES

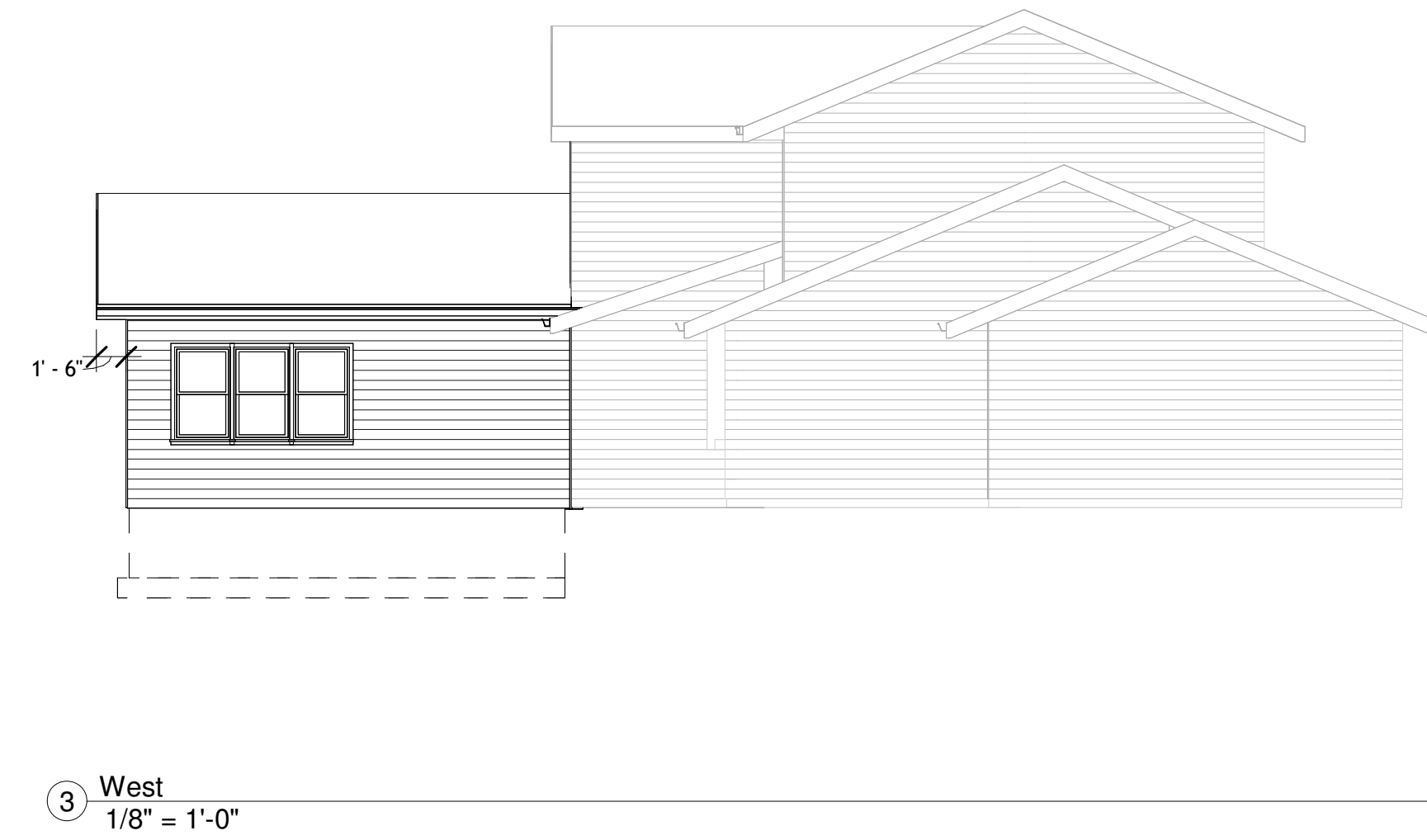


FURNITURE PLAN

A2

Revision Schedule

No.	Revision	Date



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W326S7928 Memory Ln Ct
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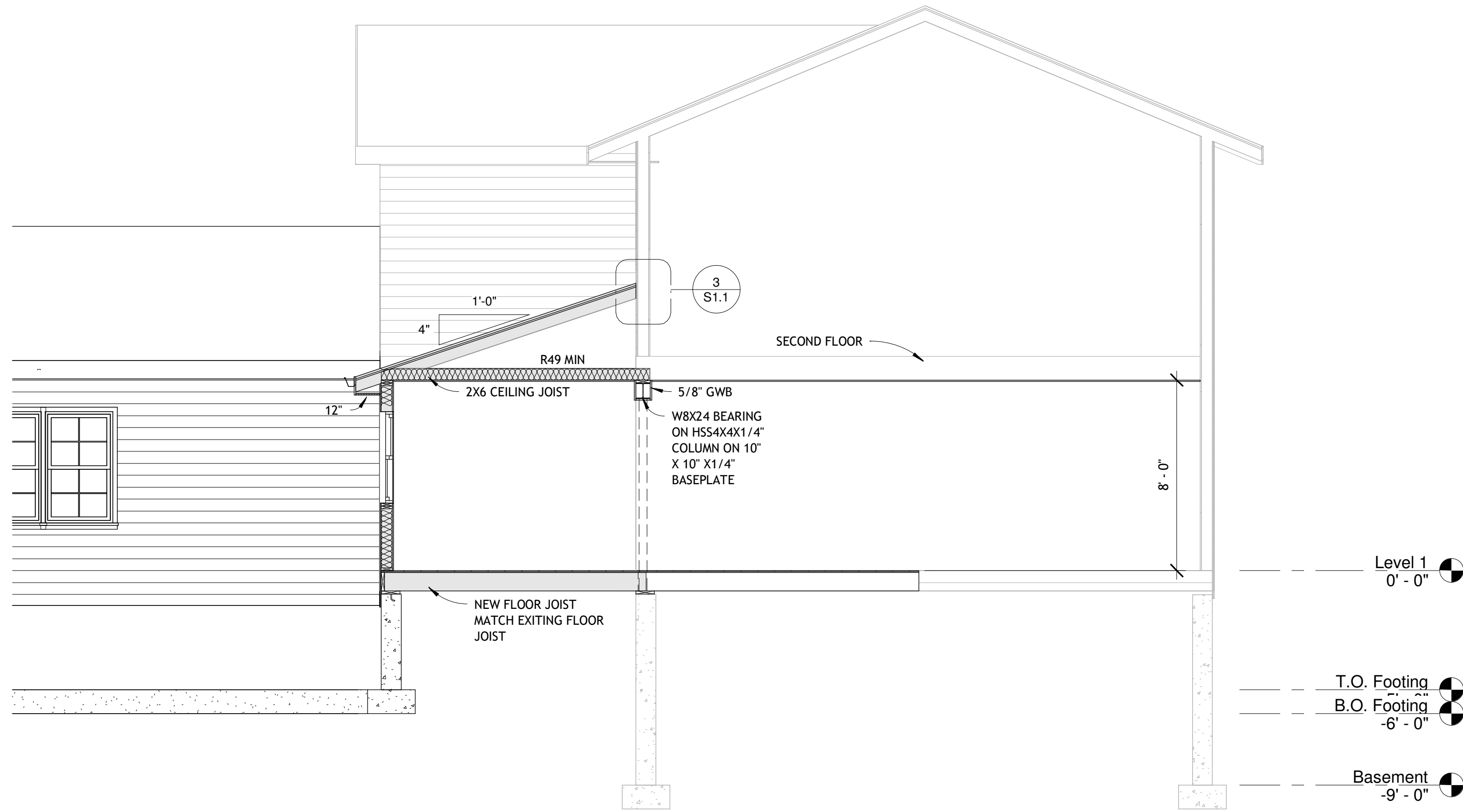
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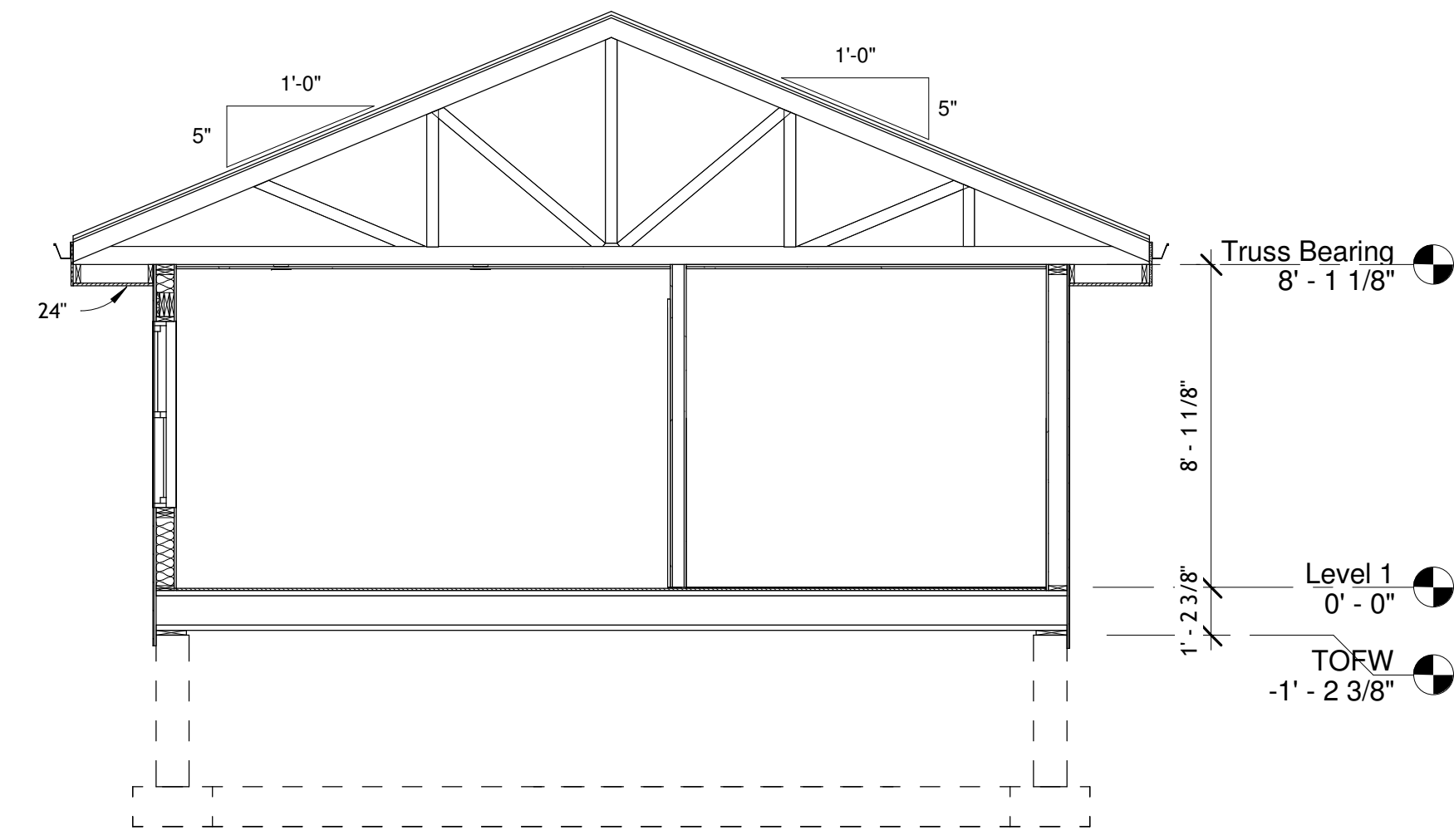
ELEVATIONS

A3

Revision Schedule		
No.	Revision	Date



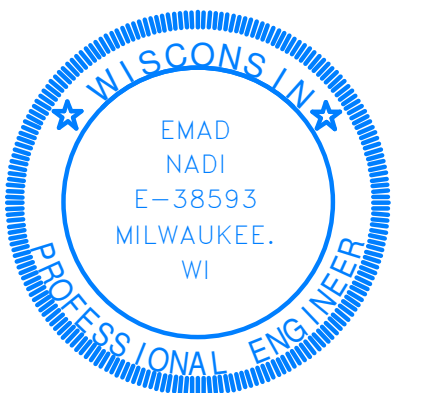
② KITCHEN FULL SECTION
1/4" = 1'-0"



① FULL SECTION
1/4" = 1'-0"

**RESIDENTIAL
ADDITION**
W326S7928 Memory Ln Ct
Mukwonago, WI 53149

SCALE
VARIES



BUILDING
SECTION

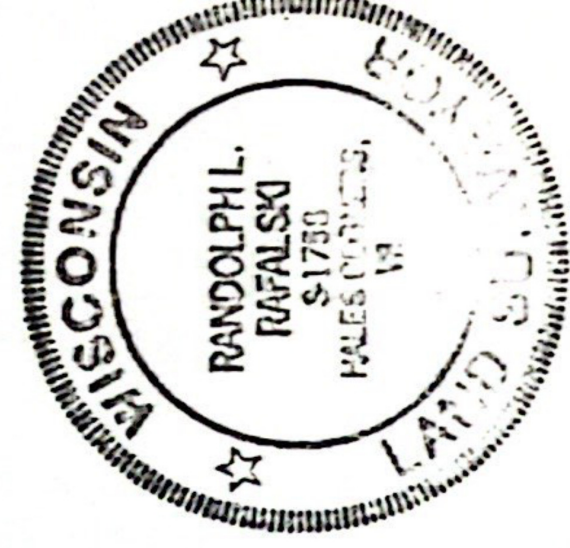
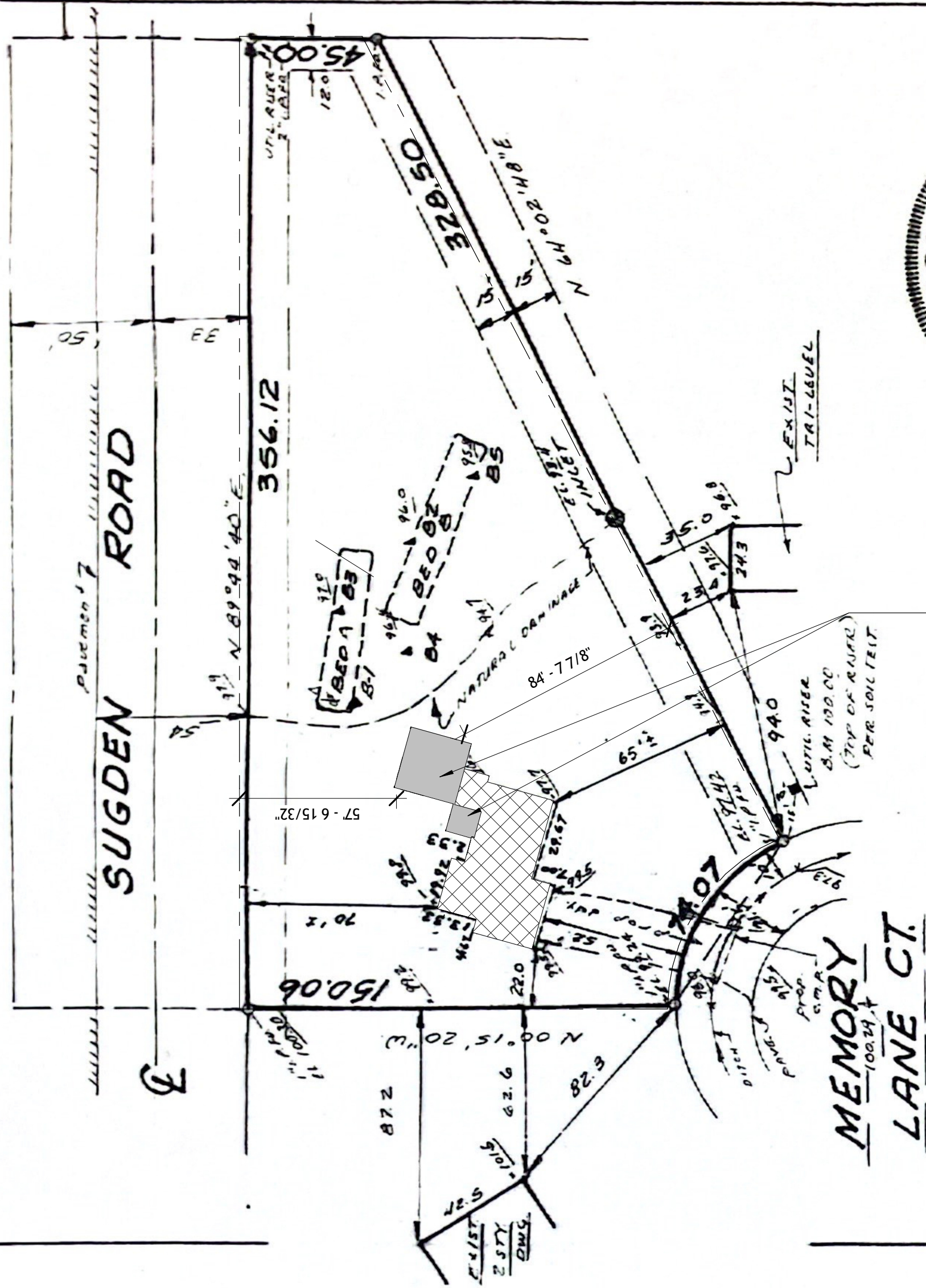
A3.1

REVISED SURVEYS FOR LYSER

Survey For
Location: **WALTER BRQS. BUILDERS**
Memory Lane Court **DYLER**

Plat of Survey

Description. LOT 6 COUNTRY BLISS, being a part of the Northwest 1/4 and the Southwest 1/4 of Section 17, Town 5 North, Range 13 East, in the Town of Mukwonago, Waukesha County, Wisconsin.



NEW 638
SQ.FT
ADDITION

NOTE
PROPOSED FINISHED YARD GRADE
= 61.100.50

INTERLINE SURVEY SERVICES, INC.
11221 W FOREST HOME AVE
HALES CORNERS WISCONSIN 53130
PHONE 425-2060

NOTE
PROP DWELLING
STAKED 10' LARGE

State of Wisconsin) ss
Milwaukee County

SURVEY CERTIFICATE Scale: 1" = 50'

We, Interline Survey Services, Inc., certify that we have surveyed the above described property and the above map is a true representation thereof, and shows the size and location of the property, its exterior boundaries, the location and dimensions of all visible structures thereon, fences, apparent easements and roadways and visible encroachments, if any.

This survey is made for the exclusive use of the present owners of the Property, and also those who purchase, mortgage, or guarantee the title thereto, within one (1) year from date hereof.

Randolph R. Rafalski
Wisconsin Registered Land Surveyor
Date: 6-20-91
Job No.: 19469

1 SITE PLAN - TN
1" = 40'-0"

Revision Schedule		
No.	Revision	Date

RESIDENTIAL ADDITION

W326S7928 Memory Ln Ct
Mukwonago, WI 53149

SCALE
VARIES



PLAT

C1.0

Revision Schedule

No.	Revision	Date
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CAST-IN-PLACE CONCRETE NOTES

- ALL WORK TO BE DONE IN ACCORDANCE WITH ACI 318 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (MOST CURRENTLY ADOPTED EDITION).
- CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER AT LEAST 48 HOURS PRIOR TO PLACING CONCRETE TO FACILITATE ON SITE OBSERVATION OF REBAR.
- ARRANGEMENT AND BENDING OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ACI DETAILING MANUAL (ACI SP-66), LATEST EDITION.
- WHEN THE AVERAGE TEMPERATURE FROM MIDNIGHT TO MIDNIGHT IS EXPECTED TO DROP BELOW 40 DEGREES FAHRENHEIT FOR THREE SUCCESSIVE DAYS, COLD WEATHER CONCRETING REQUIREMENTS SHALL BE FOLLOWED (REFER TO ACI 306R).
- WHEN AMBIENT AIR OR CONCRETE TEMPERATURES EXCEED 90 DEGREES FAHRENHEIT, STEEL REINFORCING AND/OR FORMING SURFACES ARE ABOVE 120 DEGREES, OR WHEN WIND VELOCITY, HUMIDITY, OR SOLAR RADIATION CREATE CONDITIONS OF ACCELERATED MOISTURE LOSS AND INCREASED RATE OF HYDRATION, HOT WEATHER CONCRETING REQUIREMENTS SHALL BE FOLLOWED (REFER TO ACI 305R).
- ALL CONCRETE SURFACES SHALL BE FORMED UNO OR APPROVED BY THE STRUCTURAL ENGINEER.
- CONTROL JOINTS SHALL BE PLACED IN SLAB ON GRADE AND SLAB ON METAL DECK CONSTRUCTION WITHIN 8 HOURS OF INITIAL POUR.
- CONTROL JOINTS IN NON-COMPOSITE SLAB ON METAL DECK CONSTRUCTION SHALL BE PLACED WITHIN 8 HOURS OF INITIAL POUR.
- COMPOSITE SLAB ON METAL DECK SHALL NOT HAVE SAWED CONTROL JOINTS UNLESS SPECIFICALLY IDENTIFIED, CUT WITHIN 8 HOURS IF INDICATED.
- PROVIDE WALL CONSTRUCTION JOINTS AS SHOWN IN DETAILS. ALLOW AT LEAST 24 HOURS BETWEEN POURING ADJACENT WALL SECTIONS AT CONSTRUCTION JOINTS.
- PROVIDE 1/2" ISOLATION JOINTS WHERE SLABS ABUT VERTICAL SURFACES.
- PROVIDE A 3/4" CHAMFER ON EXPOSED CORNERS OF CONCRETE UNO. TOP EDGES OF WALLS SHALL BE TOOLED UNO.
- CONCRETE COLUMN OR PIERS SHOWN INTEGRAL WITH CONCRETE WALLS SHALL BE POURED MONOLITHICALLY WITH ADJACENT CONCRETE WALLS.
- SLEEVES, CONDUITS, OR PIPES THROUGH SLABS AND WALLS SHALL BE PLACED AT THREE DIAMETERS ON CENTER, OR 4" MINIMUM.
- ALUMINUM CONDUIT OR PIPING SHALL NOT BE CAST IN CONCRETE.
- PROVIDE MINIMUM COVER PER ACI 318, 7.7.1.
- ALL LAPS SHALL BE CLASS 'B' PER ACI 318 UNLESS OTHERWISE NOTED ON THE DESIGN DRAWINGS, OR UNLESS THE DETAILER TAKES SPECIAL CARE TO PROVIDE STAGGERED LAPS. USE TOP BAR LAP LENGTHS FOR ALL HORIZONTAL WALL BARS AND FOR TOP BARS IN SLABS AND BEAMS OVER 12 INCHES DEEP.
- THE CONTRACTOR SHALL SUBMIT THE FOLLOWING:
 - CERTIFIED COPIES OF MIX DESIGNS FOR EACH CONCRETE CLASS SPECIFIED INCLUDING COMPRESSIVE STRENGTH TEST REPORTS. SUBMIT MIX DESIGNS TO THE ENGINEER 7 DAYS PRIOR TO FIRST POUR.
 - CERTIFICATION THAT MATERIALS MEET REQUIREMENTS SPECIFIED.
 - READY-MIXED CONCRETE SHALL MEET REQUIREMENTS OF ASTM C94.
 - PREPARE DESIGN MIXES FOR EACH TYPE AND STRENGTH OF CONCRETE. DESIGN CONCRETE IN ACCORDANCE WITH ACI 318, CHAPTER 5, "PROPORTIONING ON THE BASIS OF FIELD EXPERIENCE OR TRIAL MIXTURES."
 - WATER-REDUCING ADMIXTURES SHALL CONFORM WITH ASTM C494 TYPE A.
 - AIR-ENTRAINING ADMIXTURE SHALL CONFORM WITH ASTM C260.
 - PLACE CONCRETE IN COMPLIANCE WITH ACI 304.
 - CURE CONCRETE IN ACCORDANCE WITH THE RECOMMENDATION OF ACI 308.
 - CONCRETE SURFACE FINISHES
 - FOOTINGS AND FOUNDATION WALLS NOT EXPOSED TO VIEW: FORM FINISHED.
 - SURFACES EXPOSED TO VIEW: SEE ARCHITECTURAL DRAWINGS FOR FINISH INFORMATION.
 - TOPPING OVER STEEL DECK: STEEL TROWEL FINISHED.

MILD REINFORCING STEEL MINIMUM CLEAR COVER

CONCRETE CAST AGAINST EARTH AND PERMANENTLY EXPOSED TO EARTH:	
FOOTINGS:	3"
CONCRETE EXPOSED TO EARTH OR WEATHER:	
WALLS, COLUMNS, BEAMS:	
UP THROUGH #5 BARS	1 1/2"
#6 THRU #18 BARS	2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER:	
WALLS:	
UP THROUGH #11 BARS	3/4"
#14 AND #18 BARS	1 1/2"
ELEVATED SLABS:	
TOP	3/4"
BOTTOM	1"
BEAMS:	
TOP/BOTTOM/SIDE	1 1/2"
COLUMNS:	
SIDES	1 1/2"

(DIMENSIONS ABOVE ARE MINIMUMS, UNLESS NOTED OTHERWISE IN DETAILS)

CONTRACTOR NOTES

- CONTRACTOR SHALL HIRE A SHORING ENGINEER & CONTRACTOR TO DESIGN AND PROVIDE ALL SHORING REQUIRED TO SUPPORT EXISTING CONSTRUCTION AND NEW CONSTRUCTION AS REQUIRED TO BUILD THIS PROJECT.
- SHORING/OR UNDERPINNING SHALL BE DESIGNED TO LIMIT HORIZONTAL AND VERTICAL MOVEMENT OF EXISTING CONSTRUCTION TO 1/4" MAXIMUM IN ANY DIRECTION.
- IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND CONSTRUCTION SEQUENCE IN ORDER TO ENSURE THE SAFETY OF THE BUILDING AND WORKMEN DURING CONSTRUCTION (MEANS & METHODS OF CONSTRUCTION). THIS INCLUDES, BUT IS NOT LIMITED TO: SHORING, UNDERPINNING, TEMPORARY BRACING, ETC.
- FIELD VERIFY ALL DIMENSIONS & EXISTING SIZES SHOWN ON THESE CONSTRUCTION DOCUMENTS LOCATING EXISTING BUILDING ELEMENTS PRIOR TO PREPARING SHOP DRAWINGS & FABRICATING MATERIALS. GENERAL CONTRACTOR TO COORDINATE ANY CHANGES W/ ARCHITECT & ENGINEER.
- EXISTING FRAMING SHOWN ON THESE DRAWINGS IS BASED ON AVAILABLE DOCUMENTATION & FIELD OBSERVATION TO DATE. FIELD VERIFY ACTUAL DIMENSIONS/CONFIGURATIONS OF ALL STRUCTURAL MEMBERS AS NECESSARY FOR NEW CONSTRUCTION. IF SIZES DIFFER, NOTIFY ENGINEER PRIOR TO PROCEEDING WITH WORK. FIELD VERIFY ALL EXISTING MEMBER SIZES AND LOCATIONS AS REQUIRED TO PROPERLY INSTALL ALL NEW STRUCTURAL MEMBERS AS SHOWN. MODIFY AND RELOCATE ALL OTHER WORK (PLUMBING, ELECTRICAL, HVAC, ETC) AS REQUIRED TO INSTALL NEW STRUCTURAL MEMBERS AS SHOWN ON THESE DRAWINGS.

HOT WEATHER CONCRETING NOTES

- CONCRETE MIXES TO BE PLACED DURING DRY AND WINDY CONDITIONS SHALL BE MODIFIED BY THE ADDITION OF RETARDING ADMIXTURES OR SLOWER CURING CEMENT SUBSTITUTES TO MINIMIZE THE EFFECTS OF ACCELERATED CURING.
- WATER SHALL NOT BE ADDED TO CONCRETE MIXES ON SITE FOR WORKABILITY. MID OR HIGH RANGE WATER REDUCERS SHALL BE APPROVED BY STRUCTURAL ENGINEER BEFORE ADDING TO CONCRETE MIX FOR INCREASED WORKABILITY.
- INGREDIENTS USED IN CONCRETE MIXES SHALL BE COOLED TO MAINTAIN A CONCRETE TEMPERATURE BELOW 90 DEGREES FAHRENHEIT AT TIME OF PLACEMENT.
- CHILLED WATER AND CHOPPED ICE MAY BE USED IN CONCRETE MIXTURES TO CONTROL CONCRETE TEMPERATURES. AMOUNT OF CHOPPED ICE SHALL NOT EXCEED THE EQUIVALENT AMOUNT OF MIXING WATER REQUIRED FOR THE DESIGN MIX.
- RETARDING ADMIXTURES SHALL NOT BE USED IN CONCRETE MIXES WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER.

EARTHWORK NOTES

- FOR DETAILED DESCRIPTION OF EXISTING SOILS AND BORING LOGS, REFER TO REPORT OF GEOTECHNICAL EXPLORATION.
- COMPLIANCE OF SOIL COMPACTION AND MEASURES TAKEN TO ACHIEVE ALLOWABLE BEARING PRESSURE SHALL BE FIELD VERIFIED BY A QUALIFIED SOILS ENGINEER PRIOR TO PLACEMENT OF SLAB OR FOUNDATIONS.
- ALL TOPSOIL, DEBRIS, SILTS, AND ORGANIC MATERIAL SHALL BE STRIPPED AND REMOVED FROM LIMITS OF EXCAVATIONS AND EXISTING SUBGRADE SHALL BE COMPACTED TO 95% STANDARD PROCTOR MAXIMUM DRY DENSITY PRIOR TO PLACEMENT OF FILL MATERIAL.
- FILL MATERIAL SHALL BE PLACED AND COMPACTED IN LIFTS NO THICKER THAN 8". EACH LIFT SHALL MEET COMPACTION REQUIREMENTS PRIOR TO PLACEMENT AND COMPACTION OF ADDITIONAL LIFTS.
- FILL MATERIAL SHALL BE PLACED AND COMPACTED AT +1% TO -4% OPTIMUM MOISTURE CONTENT TO 95% STANDARD PROCTOR MAXIMUM DRY DENSITY, UNLESS RECOMMENDED OTHERWISE BY QUALIFIED SOILS ENGINEER.
- UNSATISFACTORY SOILS LOCATED BELOW FOUNDATIONS SHALL BE REMOVED AND REPLACED AS DIRECTED BY THE SOILS ENGINEER.

GENERAL FOUNDATION NOTES

- ANCHOR BOLTS AND/OR EMBEDMENTS SHALL BE SET TO THE FOLLOWING TOLERANCES:
TOP ELEVATION +1" TO -3/8"
OUT OF POSITION ± 1/8"
- PROTECT IN-PLACE FOUNDATIONS AND SLABS ON GRADE FROM FROST PENETRATION UNTIL PROJECT COMPLETION
- WHERE FILL MATERIAL IS REQUIRED ON BOTH SIDES OF GRADE BEAMS OR WALLS, IT SHALL BE PLACED SIMULTANEOUSLY. REFER TO PLANS AND SPECIFICATIONS FOR TYPE AND PLACING OF BACKFILL.
- WHERE FILL MATERIAL IS PLACED ON ONE SIDE OF A WALL (OR GRADE BEAM), THE WALL (OR SHALL BE ADEQUATELY SHORED AND BRACED OR THE MATERIAL SHALL NOT BE PLACED UNTIL SUPPORTING FLOOR SLABS HAVE BEEN POURED AND SET.
- REFER TO ARCHITECTURAL DRAWINGS OR PLUMBING DRAWINGS FOR SPECIFIC FLOOR DRAIN LOCATIONS & ELEVATIONS.
- COORDINATE STOOP DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

SUBMITTAL NOTES

THE FOLLOWING INFORMATION SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW. ONE ELECTRONIC (PDF) OR THREE HARD COPIES SHALL BE SUBMITTED IN A TIMELY MANNER PRIOR TO MATERIAL FABRICATION OR CONSTRUCTION TO ALLOW FOR A MINIMUM PERIOD OF TEN WORKING DAYS FOR REVIEW.

- CONCRETE MIX DESIGNS:**
- CERTIFIED COPIES OF MIX DESIGNS FOR EACH CONCRETE CLASS SPECIFIED INCLUDING COMPRESSIVE STRENGTH TEST REPORTS.

- CONCRETE REINFORCEMENT SHOP DRAWINGS:**
- SUBMIT SHOP DRAWINGS FOR FABRICATION, BENDING AND PLACEMENT OF CONCRETE REINFORCEMENT SHOWING BAR SCHEDULES, STIRRUP SPACING, DIAGRAMS OF BENT BARS, SPLICING ARRANGEMENT AND ASSEMBLIES AS REQUIRED FOR THE FABRICATION AND PLACEMENT OF CONCRETE REINFORCEMENT.

IBC 2015 TABLE 2304.10.1 MINIMUM FASTENING SCHEDULE, UNO

CONNECTION TYPE	LOCATION	FASTENING
1. JOIST TO SILL OR GIRDER.	TOENAIL	3 - 8d COMMON
2. BRIDGING TO JOIST.	TOENAIL EA END	2 - 8d COMMON
3. 1"x6" SUBFLOOR OR LESS TO EACH JOIST.	FACE NAIL	2 - 8d COMMON
4. WIDER THAN 1"x6" SUBFLOOR TO EACH JOIST.	FACE NAIL	3 - 8d COMMON
5. 2" SUBFLOOR TO JOIST OR GIRDER.	BLIND AND FACE NAIL	2 - 16d COMMON
6. SOLE PLATE TO JOIST OR BLOCKING. SOLE PLATE TO JOIST OR BLOCKING.	TYPICAL FACE NAIL AT BRACED WALL PANELS	16d @ 16" OC 3 - 16d @ 16" OC
7. TOP PLATE TO STUD.	END NAIL	2 - 16d COMMON
8. STUD TO SOLE PLATE.	TOENAIL END NAIL	4 - 8d COMMON 2 - 16d COMMON
9. DOUBLE STUDS.	FACE NAIL	16d @ 24" OC
10. DOUBLE TOP PLATES. DOUBLE TOP PLATES.	TYPICAL FACE NAIL LAP SPLICE	16d @ 16" OC 8 - 16d COMMON, U.N.O.
11. BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE.	TOENAIL	3 - 8d COMMON
12. RIM JOIST TO TOP PLATE.	TOENAIL	8d @ 6" OC
13. TOP PLATES, LAPS & INTERSECTIONS.	FACE NAIL	2 - 16d COMMON
14. CONTINUOUS HEADER, TWO PIECES.	16" OC ALONG EDGE	16d COMMON
15. CEILING JOISTS TO PLATE.	TOENAIL	3 - 8d COMMON
16. CONTINUOUS HEADER TO STUD.	TOENAIL	4 - 8d COMMON
17. CEILING JOISTS, LAPS OVER PARTITIONS.	FACE NAIL	3 - 16d COMMON, MINIMUM
18. CEILING JOISTS TO PARALLEL RAFTERS.	FACE NAIL	3 - 16d COMMON, MINIMUM
19. RAFTER TO PLATE.	TOENAIL	3 - 8d COMMON
20. 1" DIAGONAL BRACE TO EACH STUD AND PLATE.	FACE NAIL	2 - 8d COMMON
21. 1"x8" SHEATHING TO EACH BEARING WALL.	FACE NAIL	2 - 8d COMMON
22. WIDER THAN 1"x8" SHEATHING TO EACH BEARING.	FACE NAIL	3 - 8d COMMON
23. BUILT-UP CORNER STUDS.	24" OC	16d COMMON
24. BUILT-UP GIRDER & BEAMS.	FACE NAIL AT T&B STAGGERED ON OPPOSITE SIDES FACE NAIL AT ENDS & AT EACH SPLICE	20d COMMON @ 32" OC 2 - 20d COMMON
25. 2" PLANKS.	AT EACH BEARING	16d COMMON
26. COLLAR TIE TO RAFTER.	FACE NAIL	3 - 10d COMMON
27. JACK RAFTER TO HIP.	TOENAIL FACE NAIL	3 - 10d COMMON 2 - 16d COMMON
28. ROOF RAFTER TO 2x RIDGE BEAM.	TOENAIL FACE NAIL	2 - 16d COMMON 2 - 16d COMMON
29. JOIST TO BAND JOIST.	FACE NAIL	3 - 16d COMMON
30. LEDGER STRIP.	FACE NAIL	3 - 16d COMMON
31. WOOD STRUCTURAL PANELS AND PARTICLEBOARD (NAILS SPACED @ 6" OC EDGES & 12" OC FIELD): SUBFLOOR, ROOF AND WALL SHEATHING (TO FRAMING):	1/2" AND LESS 19/32" TO 3/4" 7/8" TO 1" 1 1/8" TO 1 1/4"	6d COMMON 8d OR 6d COMMON 8d COMMON 10d COMMON
SINGLE FLOOR (COVERING): ISOLATION SUBFLOOR-UNDERLAYMENT	3/4" AND LESS 7/8" TO 1" 1 1/8" TO 1 1/4"	6d DEFORMED SHANK 8d DEFORMED SHANK 10d COMMON
32. PANEL SIDING (TO FRAMING). (USE CORROSION-RESISTANT SIDING OR CASING NAIL)	1/2" AND LESS 5/8"	6d 8d
33. FIBERBOARD SHEATHING. (NAILS SPACED @ 3" OC EDGES & 6" OC FIELD)	1/2" 25/32"	6d COMMON NAIL 8d COMMON NAIL
34. INTERIOR PANELING. (NAILS SPACED @ 6" OC EDGES & 12" OC FIELD)	1/4" 3/8"	4d CASING OR FINISH 6d CASING OR FINISH

GENERAL NOTES:

- NAILING PER SCHEDULE ABOVE IS TO BE USED WHERE NAILING IS NOT SPECIFIED ON PLANS OR DETAILS. NAILING PER PLANS & DETAILS SUPERSEDES THIS NAILING SCHEDULE UNLESS APPROVED BY THE ENGINEER OF RECORD.
- COMMON OR BOX NAILS MAY BE USED EXCEPT WHERE OTHERWISE NOTED.

SHEAR WALL SCHEDULE & TYP. SHEAR TRANSFER DETAILS

SHEARWALL SYMBOL (1) (2)	SHEATHING	BLOCKING REQUIRED (3)	FASTENER SPACING	ANCHOR BOLTS (9) (10) (11)	SOLE PLATE WASHERS (4)	SOLE PLATE NAILING (11)	SHEAR CLIP OR TOE NAIL (11) (12) (13)
W1	7/16" APA RATED SHEATHING (6)	YES	6" OC EDGES & 12" OC FIELD (7)	1/2" Ø @ 3'-0" OC	3"x3" PLATE WASHER (14)	16d @ 5" OC	A35 @ 16" OC
W1	7/16" APA RATED SHEATHING (6)	YES	3" OC EDGES & 12" OC FIELD (7)	1/2" Ø @ 1'-6" OC	3"x3" PLATE WASHER (14)	16d @ 2" OC	A35 @ 6" OC
W1	5/8" GYPSUM BOARD (4)	YES	4" OC EDGES & FIELD (6)	1/2" Ø @ 4'-0" OC	STANDARD WASHER	16d @ 9" OC	A35 @ 6'-0" OC OR 10d @ 6" OC

SECTION AT SILL PLATE

KEY

- DESIGNATES EDGE NAIL SPACING, i.e. "4"-4" EDGE NAIL SPACING.
- DESIGNATES BLOCKING IS REQUIRED.
- DESIGNATES SHEATHING MATERIAL, i.e. "G"-GYPSUM BOARD, "W"-WOOD APA RATED SHEATHING.

SECTION AT FLOOR FRAMING

GENERAL NOTES:

- SEE SHEARWALL DETAILS.
- DO NOT EXCEED 16" OC STUD SPACING AT SHEARWALLS.
- WHEN BLOCKING IS REQUIRED, USE 2x BLOCKING AT ALL PANEL EDGES, SAME DEPTH AS STUD, AND PROVIDE EDGE NAILING AT ALL SUPPORTS & PANEL EDGES.
- APPLY SHEATHING PANELS EITHER VERTICALLY OR HORIZONTALLY IN 4'-0" OR WIDER SHEETS ONLY.
- APPLY SHEATHING PANELS HORIZONTALLY WITH LONG DIMENSION ACROSS STUDS IN 4'-0" OR WIDER SHEETS ONLY.
- USE 6d COOLER NAILS.
- USE 8d COMMON OR GALVANIZED BOX NAILS.
- USE 8d COOLER NAILS OR #8@ 1/2" TYPE 'S' OR 'W' DRYWALL SCREWS ONLY.
- WHERE PIT TENDONS ARE 5" OR FURTHER FROM THE TOP SURFACE OF CONCRETE, ANCHOR SILL PLATE WITH SIMPSON TITEN HD SCREW ANCHOR W/ 3x3" EMBEDMENT, SIZE AND SPACING AS SCHEDULED.
- WHERE PIT TENDONS ARE WITHIN 5" OF THE TOP SURFACE OF CONCRETE, ANCHOR SILL PLATE WITH SIMPSON A34 CLIPS PER DETAILS (WHEN APPLICABLE).
- WHEN SHEATHING APPLIED TO BOTH FACES OF STUD, REDUCE ANCHOR BOLT, SOLE, NAILING & SHEAR CLIP/TOE NAIL SPACING BY ONE HALF.
- TOE NAILS SHALL BE COMMON OR GALVANIZED BOX NAILS. TOE NAILING MUST BE FROM OUTSIDE FACE OF RIMBOARD. IF OUTSIDE FACE IS NOT ACCESSIBLE USE SHEAR CLIP OPTION.
- "A35" REFERS TO SIMPSON OR EQUAL A35 CLIP ANGLE.
- SEE ADDITIONAL REQUIREMENTS IN DETAILS.

CAST-IN-PLACE CONCRETE TOLERANCES

- CONCRETE COVER MEASURED PERPENDICULAR FROM THE SURFACE IN DIRECTION OF TOLERANCES.

MEMBERS 12" OR LESS	±3/8"
MEMBERS OVER 12"	±1/2"
- STEEL REINFORCEMENT SPACING SHALL BE WITHIN THE FOLLOWING TOLERANCES:
1/2" SPACING INSTANCES NOT TO EXCEED 1"
- PLACEMENT OF EMBEDDED ITEMS SHALL BE WITHIN THE FOLLOWING TOLERANCES:
VERTICAL ALIGNMENT ±1"
LATERAL ALIGNMENT ±1"
LEVEL ALIGNMENT ±1"
- PLACEMENT OF FOOTINGS SHALL BE WITHIN THE FOLLOWING TOLERANCES:
LATERAL ALIGNMENT ±2"
LEVEL ALIGNMENT ±1/2" TO -2"
(LEVEL ALIGNMENT SUPPORTING MASONRY) ±1/2"
- CROSS-SECTIONAL DIMENSION OF FOUNDATIONS SHALL BE WITHIN THE FOLLOWING TOLERANCES.

SPREAD FOOTINGS / PILE CAPS	±2" TO -1/2"
FOUNDATION THICKNESS	±5%
- TOP OF FOOTING SLOPE 1" IN 10'

REINFORCING NOTES

- REINFORCING SHALL BE DETAILED IN ACCORDANCE WITH ACI 315 (CURRENT EDITION), ARRANGEMENT AND BENDING OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ACI DETAILING MANUAL (ACI SP-66), LATEST EDITION.
- ALL LAPS SHALL BE CLASS 'B' PER ACI 318 UNLESS OTHERWISE NOTED ON THE DESIGN DRAWINGS, OR UNLESS THE DETAILER TAKES SPECIAL CARE TO PROVIDE STAGGERED LAPS. USE TOP BAR LAP LENGTHS FOR ALL HORIZONTAL WALL BARS AND FOR TOP BARS IN SLABS AND BEAMS OVER 12 INCHES DEEP.
- LAP LENGTH SHALL BE SPECIFICALLY NOTED ON PLACING DRAWINGS WHERE MORE THAN ONE BAR MAKES UP A CONTINUOUS STRING.
- CORNER BARS WITH CLASS 'B' PER ACI 318 LAPS SHALL BE PROVIDED AT ALL WALL CORNERS AND INTERSECTIONS PER DETAIL 1S200.
- HORIZONTAL BARS, EXCEPT FOR CONTINUOUS STRINGS FROM ONE CORNER OF OPENING TO ANOTHER, SHALL BE DETAILED TO SHOW THE DISTANCE FROM AT LEAST ONE END OF THE BAR TO THE NEAREST BUILDING GRID LINE OF WALL.
- WELDED WIRE FABRIC SHALL BE LAPPED AND/OR ANCHORED TO DEVELOP Fy PER ACI 315.
- PROVIDE MINIMUM COVER PER ACI 318, 7.7.1.
- PROVIDE ISOLATION BOARD WHERE SLABS ABUT VERTICAL SURFACES AS SHOWN IN DETAIL 4S20.
- WIRE SPACERS, CHAIRS, TIES, ETC., FOR SUPPORT OF STEEL REINFORCING SHALL BE PROVIDED BY THE CONTRACTOR TO ENSURE REINFORCING IS PLACED IN THE PROPER POSITION DURING CONCRETE PLACEMENT.
- STEEL REINFORCING SPLICES OF ADJACENT BARS SHALL BE STAGGERED SUCH THAT SPICES ARE 4 FEET APART, MINIMUM.
- ALL HOOKS IN STEEL REINFORCING SHALL BE ACI STANDARD HOOKS, UNLESS NOTED OTHERWISE IN CONSTRUCTION DOCUMENTS.
- WELDING OF STEEL REINFORCING IS NOT PERMITTED.

WOOD STRENGTHS:
WOOD MEMBERS USED FOR THIS PROJECT SHALL BE OF THE FOLLOWING GRADES/STRENGTHS AT A MINIMUM. HIGHER GRADES INDICATED IN PLANS OR SCHEDULES GOVERN OVER THESE MINIMUMS.

DIMENSIONAL LUMBER:
EXTERIOR WALL STUDS SPECIES GRADE STRENGTH PROPERTIES

2 x 6	SPRUCE PINE FIR No.1No.2
E = 1,400 ksi	Fb = 875 psi
Fc = 1,150 psi	

INTERIOR WALL STUDS SPECIES GRADE STRENGTH PROPERTIES

2 x 4 (UNO)	SPRUCE PINE FIR No.1No.2
E = 1,400 ksi	Fb = 875 psi
Fc = 1,150 psi	

INTERIOR BEAMS, JOISTS, AND HEADERS SPECIES GRADE STRENGTH PROPERTIES

D-F-L	No.1 & BETTER
E = 1,800 ksi	Fb = 1,200 psi
Fv = 180 psi	Fc (papp) = 625 psi

EXTERIOR BEAMS AND JOISTS SPECIES GRADE STRENGTH PROPERTIES

SOUTHERN PINE (PRESSURE TREATED) No.1	E = 1,800 ksi
Fb = 1,000 psi	Fv = 175 psi
Fc (papp) = 625 psi	

LAMINATED VENEER LUMBER:

E = 2,000 ksi	Fb = 2,800 psi
Fv = 285 psi	Fo(para) = 750 psi
Fo(perp) = 2,510 psi	

ROOF TRUSS SUBMITTAL NOTES

NOTE: TRUSS MANUFACTURER MAY NOT DEVIATE FROM THE FRAMING PLANS UNLESS PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER HAS BEEN GIVEN. IT IS THE TRUSS MANUFACTURER'S RESPONSIBILITY TO SEEK SUCH APPROVAL PRIOR TO MANUFACTURE AND INSTALLATION OF FRAMING MEMBERS.

- WOOD TRUSS SHOP DRAWINGS SHALL SHOW THE FOLLOWING INFO:**
- INFORMATION WHICH THE RESPONSIBLE BUILDING DESIGN PROFESSIONAL WILL CHECK FOR COMPLIANCE WITH CONTRACT DOCUMENTS:
 - ERECTION PLAN: SHOWING DIMENSIONED LOCATIONS AND TRUSS IDENTIFICATION.
 - BEARING DETAILS: SHOWING BEARING LENGTH, WIDTH, AND DEPTH INDICATING CONFORMANCE TO DESIGN CALCULATIONS.
 - DESIGN LOADS: ALL DEAD AND LIVE LOADS SHALL BE SHOWN ON THE FRAMING PLAN OR TRUSS ELEVATION INDICATING CONFORMANCE TO TRUSS CALCULATIONS.
 - ALL PERMANENT BRACING: SHOW TOP CHORD, BOTTOM CHORD, & WEB MEMBER BRACING ON FRAMING PLAN AND TRUSS ELEVATION. SUPPLIER AND INSTALLER OF THIS BRACING SHALL ALSO BE INDICATED.
 - TRUSS DIMENSIONS: SHOW DEPTH, SPAN BEARING, HEIGHT, AND SLOPES AT ALL CRITICAL POINTS.
 - TRUSS DEFLECTIONS: SHOW LIVE LOAD AND TOTAL LOAD DEFLECTION BASED UPON DESIGN LOADS.
 - INFORMATION THAT SHALL BE THE RESPONSIBILITY OF THE FABRICATOR AND TRUSS DESIGNER AND SHALL BE PROVIDED WITH THE SHOP DRAWING SUBMITTAL
 - MEMBER DESIGN: INCLUDING WEB CONFIGURATION, MEMBER SIZE, GRADE OF LUMBER, FABRICATED SPLICES, AND MEMBER BRACING REQUIRED BY TRUSS DESIGN.
 - MEMBER CONNECTIONS: DESIGN AND INDICATE ALL NECESSARY HARDWARE FOR PROPER INSTALLATION OF TRUSSES INCLUDING, BUT NOT LIMITED TO: GIRDER PLY CONNECTIONS, TRUSS-TO-GIRDER CONNECTIONS, TIE-DOWNS, AND FIELD SPLICES.
 - INTERIOR CONNECTIONS: DESIGN AND SHOW DETAIL OF WEB AND CHORD CONNECTIONS, INCLUDING CONNECTOR PLATE SIZES, CAPACITIES, AND BOLT SIZES.
 - ERECTION PLAN: SHOW SPACING AND LAYOUT OF ANY TEMPORARY BRACING REQUIRED FOR ERECTION.
 - STRUCTURAL DESIGN OF TRUSSES: SUBMIT COMPLETE TRUSS CALCULATIONS STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER IN THE STATE OF WISCONSIN AND OBTAIN ALL APPROVALS NECESSARY FOR CONFORMANCE TO BUILDING CODE. VERIFY SUBMITTAL AND APPROVAL BY SENDING A COPY TO THE BUILDING DESIGN PROFESSIONAL.
 - PROVIDE CONTRACTOR/INSTALLER WITH ALL DATA NECESSARY FOR PROPER INSTALLATION.

WOOD FRAMING NOTES

(APPLY TO ALL FRAMING DRAWINGS)

- ARCHITECT AND CONTRACTOR SHALL DETAIL AND CONSTRUCT BUILDING FINISHES TO ACCOMMODATE 3/4" TO 3/8" OF EXPECTED WOOD SHRINKAGE PER FLOOR OF WOOD CONSTRUCTION. LUMBER SHALL NOT BE STORED IN STANDING WATER AND INSTALLED LUMBER THAT HAS BEEN EXPOSED TO THE ELEMENTS SHALL BE DRIED SUFFICIENTLY BEFORE INTERIOR OR EXTERIOR FINISHES ARE APPLIED TO AVOID CRACKING/CRUSHING.
- FLOOR SHEATHING: 3/4" TONGUE AND GROOVE APA RATED WOOD FLOOR SHEATHING (PLYWOOD OR OSB). GLUE AND NAIL TO FLOOR FRAMING WITH 8d (2x) COMMON OR BOX NAILS @ 6" O.C. ALONG PANEL EDGES AND 12" O.C. ALONG INTERMEDIATE MEMBERS.
- ROOF SHEATHING: 3/4" TONGUE AND GROOVE APA RATED WOOD SHEATHING (PLYWOOD OR OSB). ATTACH SHEATHING TO ROOF FRAMING MEMBERS WITH 2x(2) COMMON OR BOX NAILS @ 6" O.C. ALONG PANEL EDGES 12" O.C. ALONG INTERMEDIATE MEMBERS. DISTANCE OF NAILS FROM EDGE OF SHEATHING SHALL BE 3/4" STAGGER ALL SHEATHING JOINTS.
- EXTERIOR WALL SHEATHING: 7/16" OSB SHEATHING OR FIBERGLASS GYP. ATTACH DIRECTLY TO EXTERIOR FACE OF STUD WALL WITH 8d (2x) COMMON OR BOX NAILS @ 6" O.C. ALONG PANEL EDGES AND 12" O.C. ALONG INTERMEDIATE MEMBERS.
- INTERIOR WALL SHEATHING: SEE SHEAR WALL SCHEDULE FOR SHEATHING TYPE AND ATTACHMENT INFORMATION FOR ALL WALLS DESIGNATED ON THE PLANS AS A SHEAR WALL. FOR ALL OTHER INTERIOR LOAD BEARING WALLS NOT DESIGNATED AS SHEAR WALLS, PROVIDE A MINIMUM OF (1) LAYER OF 5/8" GYPSUM BOARD ATTACHED TO WALL STUDS WITH #6 x 1 1/4" TYPE "S" OR "W" DRYWALL SCREWS @ 7" OC ALONG PANEL EDGES AND INTERMEDIATE MEMBERS.
- ALL CONNECTIONS SHALL COMPLY WITH IBC 2015 TABLE 2304.10.1 FASTENING SCHEDULE. SEE SHEET S002. DETAILS IN DRAWINGS WITH MORE EXTENSIVE CONNECTIONS SHALL GOVERN OVER THOSE SHOWN IN TABLE.
- AT INTERIOR BEARING WALLS WHERE FLOOR TRUSSES BEAR ON THE WALL FROM BOTH SIDES, LAP TRUSSES SO THAT EACH TRUSS BEARS ON FULL WIDTH OF WALL.
- PROVIDE CROSS BRIDGING BETWEEN FLOOR JOISTS PER 2005 NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION 4.4.1
- FOR ALL WOOD MEMBERS THAT FRAME INTO OTHER MEMBERS AND WHERE NOT SPECIFICALLY DETAILED OR SPECIFIED IN THE DRAWINGS, USE AN APPROPRIATE SIMPSON STRONG TIE HANGER SELECTED FOR ACTUAL END REACTION. CONTACT ENGINEER FOR END REACTION IF NECESSARY.
- ANCHOR EACH 2x4 PURLIN TO EACH TRUSS W/ (2) 8d NAILS-TOE NAILLED

RESIDENTIAL ADDITION
W326S7928 Memory Ln Ct
Mukwonago, WI 53149

SCALE VARIES

STRUCTURAL NOTES

S1.0

Revision Schedule		
No.	Revision	Date

Sheath roof with 3/4 APA rated PLY (Grade 32/16) w/ 10d nails @ 6" o/c edges, 12" o/c field. Minimum the values in the table above

Walls:	Field	Windward	23.08
	Edge	Leeward	-25.31
		Windward	23.08
		Leeward	-28.33
Roof:	Zone 1 (Field)	Windward	9.41
		Leeward	-26.74
	Zone 2 (Edge)	Windward	9.41
		Leeward	-31.7
	Zone 3 (Corner)	Windward	9.41
		Leeward	-31.7
Overhang:	Roof Edge		-27.24 (total, both surfaces)
	Roof Corner		-19.81
Parapet Windward side (case A)	Wall Field		47.01
	Wall Edge		
Leeward side (case B)	Wall Field		42.84
	Wall Edge		46.28

SNOW LOADS

GROUND SNOW LOAD: 35.00 PSF
FLAT-ROOF SNOW LOAD: 29.40 PSF
SNOW EXPOSURE FACTOR: 1.00
SNOW IMPORTANCE FACTOR: 1.00
THERMAL FACTOR: 1.20
DRIFT SURCHARGE LOAD: 0.00 PSF
WIDTH OF SNOW DRIFT: 0.00 FT

WIND LOADS

DESIGN WIND SPEED: 115.00 MPH
RISK CATEGORY: II
WIND EXPOSURE: B

MWFRS Wind Calculations
MWFRS loads are calculated using the provisions of ASCE 7-10 Chapter 28. Loads are first calculated on the structure as a whole, for transmission to shear walls.
Common Values

The following values are common for the entire structure:
V = 115.00 mph (basic wind speed, as entered by user)
K_r = 0.85 (wind directionality factor, from Table 26.6-1, for Main Wind Force Resisting System)

K_t = 1.00 (topography factor, as entered by user, from Table 26.8-1)
K_z = 0.70 (velocity pressure coefficient, from Table 28.3-1 Note 1, evaluated at roof mean height)

Common Velocity Pressure
Velocity pressure at roof mean height (q_r), Equation 28.3-1 evaluated at roof mean height per 28.4.1:
q = h 0.00256k = zkzkdV² 0.00256 0.70 1.00 0.85 115.00 mph 2 = 24.76 psf

Force on Roof

Wind pressures on the roof are calculated here and will be used later when distributing load to the loadbearing walls that support the roof.
GC_r Coefficient Determination

Values from Figure 28.4-1 for roof zones, taking worst case of Load Case A and B:

GC_{r1} = -1.07 (Windward surface, edge zone)
GC_{r2} = -0.69 (Windward surface, field zone)
GC_{r3} = -0.53 (Leeward surface, edge zone)
GC_{r4} = -0.37 (Leeward surface, field zone)

Design Pressures

Pressure values from Equation 28.4-1:
p = q_r (GC_{r1} - GC_{r2}) = (20.16 psf)(-1.07-0.18) = -25.20 psf (Windward surface, edge zone)
p = q_r (GC_{r1} - GC_{r3}) = (20.16 psf)(-1.07-0.53) = -17.54 psf (Windward surface, field zone)
p = q_r (GC_{r3} - GC_{r4}) = (20.16 psf)(-0.53-0.18) = -14.31 psf (Leeward surface, edge zone)
p = q_r (GC_{r3} - GC_{r4}) = (20.16 psf)(-0.53-0.18) = -11.09 psf (Leeward surface, field zone)

These pressures are applied normal to the roof. For sloped roofs, only the vertical component will be taken when distributing pressures to walls.

GRAVITY LOADS

ROOF LIVE LOAD: 25.00 PSF
FLOOR LIVE LOAD: 100.00 PSF
SNOW LOAD : 30 PSF

EARTHQUAKE LOADS

RISK CATEGORY: II
CLASS: B
SEISMIC IMPORTANCE FACTOR: 1.00
MAPPED 0.2 SECOND SPECTRAL RESPONSE ACCELERATION: 0.200
MAPPED 1.0 SECOND SPECTRAL RESPONSE ACCELERATION: 0.050
DESIGN 0.2 SECOND SPECTRAL RESPONSE ACCELERATION: 0.160
DESIGN 1.0 SECOND SPECTRAL RESPONSE ACCELERATION: 0.040
SEISMIC DESIGN CATEGORY: A
LATERAL FORCE RESISTING SYSTEM: ORDINARY REINFORCED MASONRY SHEAR WALLS
DESIGN BASE SHEAR: 4.22 K
SEISMIC RESPONSE COEFFICIENT: 0.08
RESPONSE MODIFICATION FACTOR: 2.00
SEISMIC ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE METHOD

SOIL CAPACITY : ASSUMED 3000 PSF

MASONRY DESIGN CRITERIA

DESIGN STANDARD: TMS 402-13
SPECIFIED COMPRESSIVE STRENGTH OF MASONRY (FM): 2,000.00 PSI
GRADE OF REINFORCEMENT (FY): 60,000.00 PSI
MASONRY UNIT: 8 IN CMU
CMU DENSITY: NORMALWEIGHT
MASONRY MORTAR TYPE: TYPE S PORTLAND CEMENT/LIME

CONCRETE

DESIGN STANDARD: AISCE
SLAB ON GRADE : f_c = 4000 psi
FOOTING : f_c = 3000 psi

STEEL

WIDE FLANGE "W SHAPE" : A992
PLATES AND OTHER : A36
BOLTS: A325

WELDS:

WELDED CONNECTIONS
ELECTRODES: 70 KSI

GENERAL NOTES:
1. ROOF TRUSS SCHEDULE DENOTES TRUSS SPACING, DEPTH, BEARING CONDITIONS AND LOADING.
INDIVIDUAL TRUSS LENGTHS TO BE VERIFIED BY TRUSS MANUFACTURER.
2. SEE PLANS FOR VARIATIONS IN TRUSS BEARING CONDITIONS.
3. CONCENTRATED LOADS DUE TO DOOR / WINDOW JAMBS ARE NOT EXPLICITLY PROVIDED AND SHALL BE CALCULATED BY TRUSS DESIGNER.
4. TRUSS MANUFACTURER TO DESIGN ALL TRUSSES BASED ON GIVEN LOADING.
5. ALL TRUSSES TO HAVE PITCHED TOP CHORDS & FLAT BOTTOM CHORDS.
6. SEE ARCHITECTURAL DRAWINGS FOR SECTIONS SHOWING TRUSS PROFILES, BEARING ELEVATIONS, AND PITCH.

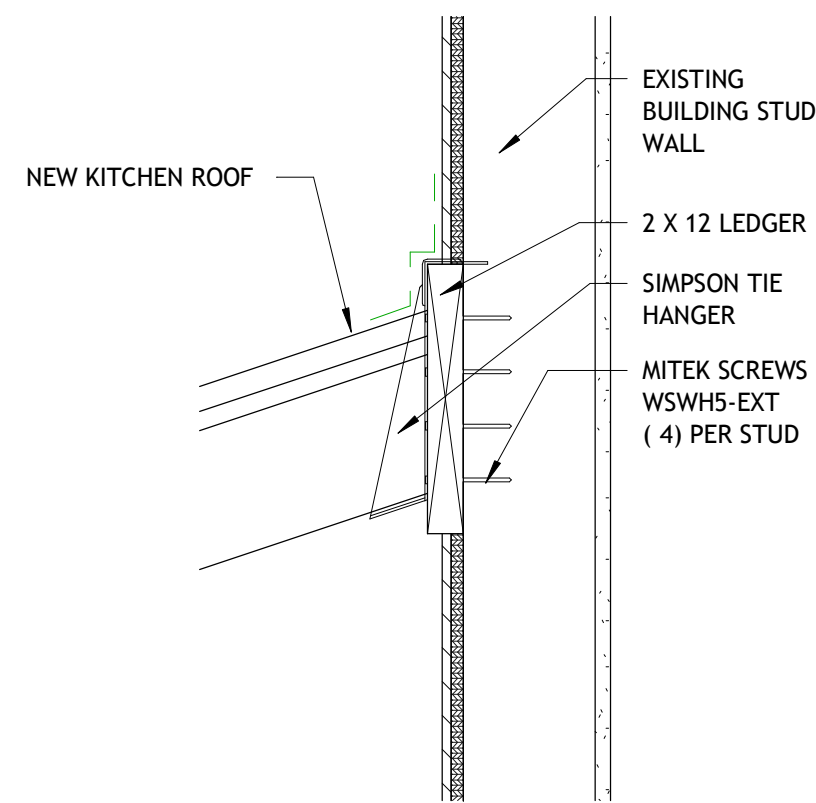
1. TRUSS MANUFACTURER MAY NOT DEVIATE FROM THE FRAMING PLANS UNLESS PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER HAS BEEN GIVEN. IT IS THE TRUSS MANUFACTURER'S RESPONSIBILITY TO SEEK SUCH APPROVAL PRIOR TO MANUFACTURE AND INSTALLATION OF FRAMING MEMBERS.
2. ROOF TRUSS DESIGNER TO VERIFY MINIMUM DESIGN LOADS.
3. DESIGN UPLIFT ON ROOF TRUSSES AS INDICATED IN THE DESIGN DATA. PROVIDE A TIE DOWN CLIP AT EACH TRUSS, AT EVERY POINT OF BEARING.

DEFLECTION LIMITS:
LIVE LOAD L/360
TOTAL LOAD L/240 (MAX TOTAL 1")

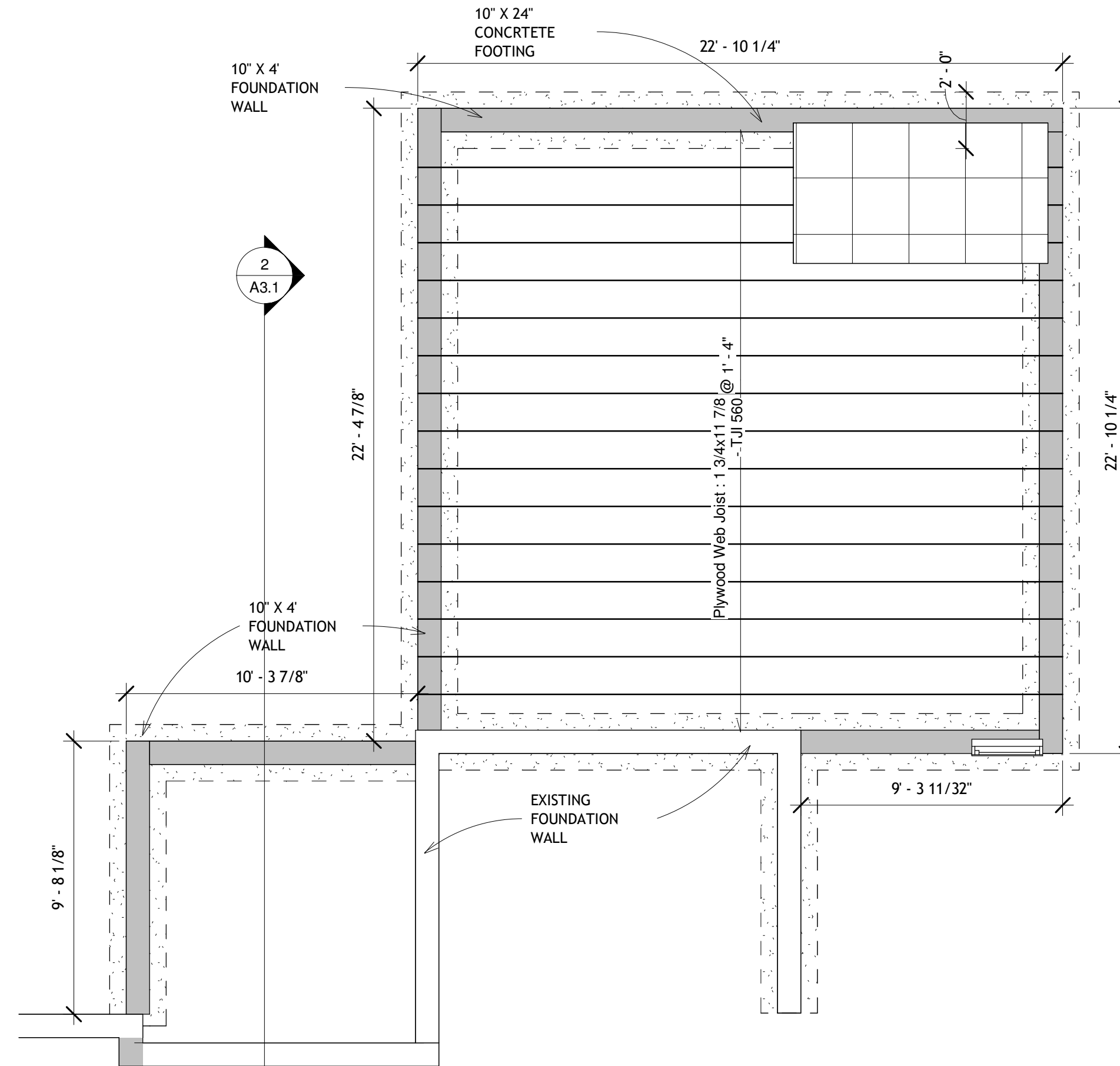
LOADS:
ROOF DEAD LOAD SEE TRUSS SCHEDULE

ROOF WIND LOAD (ALSO SEE DESIGN DATA FOR ADDITIONAL WIND LOADS)
DESIGN/BALANCED SNOW LOAD (P_s) SEE DESIGN DATA

Fire-retardant-treated wood framing and sheathing complying with Section 2303.2 shall be permitted within exterior wall assemblies of a 2-hour rating or less.



③ KITCHEN ROOF LEDGER DETAILS
1 1/2" = 1'-0"



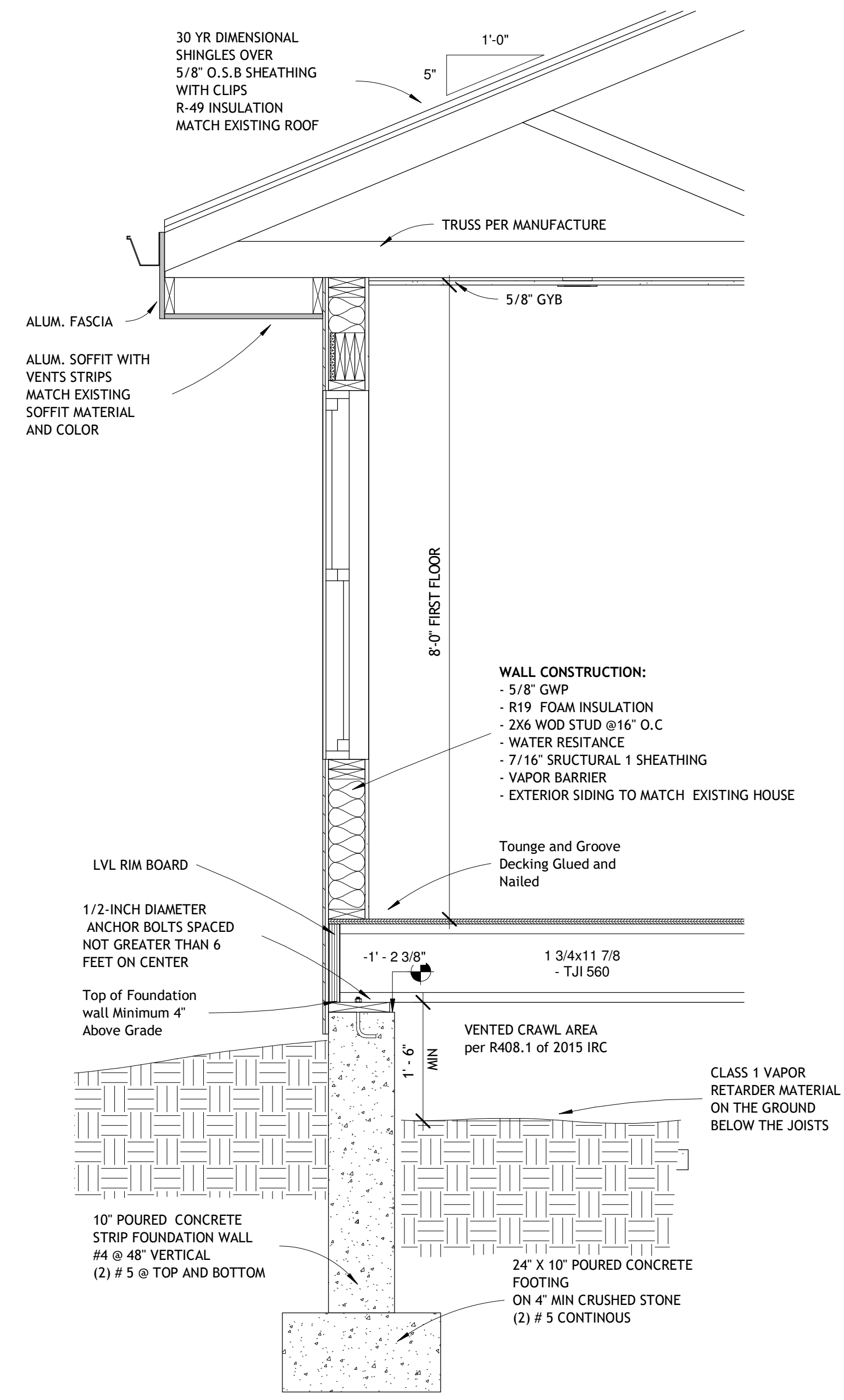
② TOFW
1/4" = 1'-0"

HOLD-DOWN SCHEDULE

MARK	TYPE	ANCHOR BOLT SIZE	MIN END POST
HD-1	HDU8-SDS2.5	1"	(3) 2x6 SPF No.1/No.2

HOLD-DOWNS TO BE PLACED AT THE CORNERS OF THE NEW ADDITION -

④ HOLD-DOWN
1 1/2" = 1'-0"



① WALL SECTION
3/4" = 1'-0"

**RESIDENTIAL
ADDITION**

W326S7928 Memory Ln Ct
Mukwonago, WI 53149

SCALE
VARIES



**STRUCTURAL
PLAN**

S1.1